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DISTRICT OF MISSION

HYDROGEOLOGICAL INVESTIGATION FOR GROUNDWATER SUPPLY MIRACLE VALLEY, B.C.

Prepared by

PITEAU ASSOCIATES ENGINEERING LTD.

PROJECT 3131

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EXECUTIVE SUMMARY

Piteau Associates Engineering Ltd. has been retained by the District of Mission to conduct hydrogeologic investigations at the Miracle Valley to explore the feasibility of supplying up to 210 L/s of quality groundwater for municipal water supply. Our investigations have included the construction and testing of two 200mm (8") diameter test wells – TW11-1 at the south end of Burns Road, and TW12-1 at the north end of Stave Lake Road.

The Miracle Valley Aquifer is a 10 km² sand and gravel aquifer that is confined by a thick sequence of clay and sandy till. Primary sources of recharge include exfiltration from watercourses along the east side of the valley and downward infiltration of incident precipitation. Groundwater flow is interpreted to be northward above Hartley Road towards Stave Lake. South of Hartley Road, groundwater flows to the south/southwest and discharges to a number of spring-fed creeks. The results of aquifer pumping tests conducted with TW11-1 and TW12-1 indicate that aquifer sediments are highly permeable, and theoretical short-term yields for larger diameter (12" to 16") pumping wells constructed in these areas are 124 and 360 L/s, respectively. It therefore appears possible to extract groundwater at 210 L/s from two or more wells at either location.

Groundwater quality measured in water samples collected from TW11-1 is excellent, and the concentrations of all constituents analyzed were within Guidelines for Canadian Drinking Water Quality. However, analyses conducted on shallower and deeper groundwater samples in the vicinity indicate that iron concentrations could exceed aesthetic objectives at horizons in the aquifer. Concentrations of total manganese and lead in groundwater samples collected from TW12-1 slightly exceeded drinking water quality guidelines.

A long-term groundwater withdrawal of 210 L/s in the vicinity of TW11-1 is expected to reduce flows in creeks draining the south portion of the aquifer. As fish habitat is considered to be

already compromised along some reaches, obtaining environmental approval could be challenging. In the vicinity of TW12-1, project withdrawals are not expected to impact these creeks, as groundwater flow is interpreted to be northward toward Stave Lake. However, other watercourses may potentially be affected. Groundwater withdrawals of 210 L/s in the vicinity of TW11-1 or TW12-1 are not expected to affect the performance of other wells identified in the near vicinity. This should be verified on a well-to-well basis based on information regarding the well's depth and current use.

In accordance with the British Columbia *Environmental Assessment Act*, an environmental assessment will be required for groundwater extraction projects with an anticipated withdrawal rate exceeding 75 L/s. The scope of assessment should be determined by an environmental consultant, with input from the Environmental Assessment Office, and may include:

- Completing a survey of existing wells and operating springs at potential municipal well sites;
- Establishing a seasonal baseline of flows and water quality in nearby watercourses draining the aquifer;
- Continuation of long-term monitoring of piezometric levels in the aquifer; and/or
- Conducting extended, high-rate (75 to 100 L/s) aquifer pumping test(s) with larger diameter (12 to 16") test production wells, and/or developing a numerical groundwater flow model to estimate aquifer response to project withdrawals.

More specific recommendations have also been provided to examine the water supply prospects in the north end of the Miracle Valley near Stave Lake to investigate the extent of the aquifer in this direction, degree of hydraulic connection with Stave Lake, and ultimately to supply a much greater quantity of groundwater (e.g., 1,000 L/s). These include reviewing any borehole information available for the BC Hydro right-of-way, determining land ownership and reconnoitering the lands to the north of the power line right-of-way to identify possible drilling sites as close as possible to the lake high water level, drilling a test well, and conduct aquifer pumping tests. If results are favourable, additional work would be needed to assess the aquifer capacity in this area in support of an environmental assessment. The scope of these investigations would be generally similar to those described above.



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1. INTRODUCTION

1.1 BACKGROUND AND OBJECTIVES

Piteau Associates Engineering Ltd. (Piteau) was retained by the District of Mission (DOM) to conduct hydrogeological investigations in the Miracle Valley (the Valley), located approximately 10 km north of the Mission town centre (Fig. 1). The purpose of this work was to assess whether groundwater supply development in the Valley could be a viable means of meeting future water demand. This would require a source capable of supplying on the order of 18 ML/day (210 L/s) of potable fresh water.

1.2 SCOPE OF WORK

This investigation was conducted between November 2011 and March 2012. Over this period, Piteau carried out the following tasks:

- Reviewing existing information on groundwater extraction and exploration in the Valley (water well records, consulting reports);
- Collecting relevant mapping information (topography, surficial geology, bedrock geology) and spatial (GIS) data;
- Reviewing stereo-paired aerial photographs of the Valley;
- Obtaining information on potential groundwater pollution hazards in the Valley, including a search of the BC Ministry of Environment (BC MOE) Site Registry;
- Selecting two locations for the drilling of test production wells in consultation with the DOM;

- Conducting preparatory steps before drilling, such as obtaining land access permits, checking for underground services, co-ordinating a professional survey to verify lot boundaries, and arranging the construction of drill pads;
- Retaining and supervising a drilling contractor to construct test wells at two locations;
- Retaining and supervising a pump service contractor to conduct aquifer pumping tests with the new test wells;
- Collecting groundwater samples for water quality analyses;
- Retaining an environmental consultant to evaluate aquatic ecosystem components within watercourses and riparian areas that could be potentially impacted by future large-scale groundwater supply development;
- Establishing two surface water monitoring stations, and installing automated monitoring
 instrumentation at these stations and at the two test wells to obtain a long-term record of
 water level variations. An electronic copy of these data are provided on a CD included
 with hardcopies of this report;
- Analyzing relevant hydrogeologic information and analyzing the results of well drilling and testing program to provide statements pertaining to potential groundwater supply development in the Miracle Valley. These statements and our supporting analysis are presented in this report.

2. BACKGROUND INFORMATION

2.1 GEOGRAPHIC SETTING

The Miracle Valley, sometimes referred to as the Upper Hatzic Valley, is located on a topographic bench north of the Hatzic Valley (Fig. 2). It extends from Lagace Creek at its south end to the Stave Lake reservoir at its north end, and is bounded by steep mountainous terrain on the east and west sides. Ground elevations rise abruptly north of Durieu Road from 20 to 100 metres above sea level (m-asl), then continue to rise gradually to 140 m-asl at Hartley Road. Continuing northward, ground elevations decline gradually to about 80 m-asl at Stave Lake.

Land use in the Valley is predominantly forest, followed by rural residential and low-intensity agriculture. At the south end of the Valley, there are two fish hatcheries, one at 35745 Durieu Road, and the other at 12451 Stave Lake Road (Miracle Valley Springs). A rock quarry (Stave Lake Quarry) is located at the north end of the valley at 13361 Stave Lake Road. A BC Hydro power corridor runs southwest to northeast across the north end of the Valley.

2.2 CLIMATE

The nearest climate monitoring station is the Mission West Abbey, located about 8 km southwest of the Valley at an elevation of 221 m-asl. Data released by Environment Canada for the period January 2000 to October 2011 indicate that this station receives 1,808mm of precipitation annually. Average monthly precipitation amounts range from 54mm in July and 274mm in January. Approximately 68% of the total annual precipitation falls between the months of October and March.

2.3 GEOLOGY

Bedrock mapping by Roddick (1965) indicates that the Valley is bounded by Mesozoic and Paleozoic plutonic bedrock (quartz diorite) belonging to the Coast Plutonic Complex.

During the last ice age that began approximately 25,000 years ago, the Fraser Lowland was repeatedly invaded by glaciers from the adjacent Coast Mountains. Sediments deposited by these processes have been extensively mapped by Armstrong (1984, 1990). Periods of glacial

advance and retreat brought deposits of till and glaciofluvial sediments, which were overlain by fluvial, marine, and colluvial deposits during non-glacial periods. During the peak of glacial advance approximately 13,000 years ago, glaciers had carved out a U-shaped valley and depressed the land surface by about 80m below current sea level. Marine waters inundated some valleys, leaving thick deposits of glaciomarine clay and silt. During glacial retreat, meltwater channels and shrinking lobes of ice left glaciofluvial and ice contact sands and gravels in some areas. More recently, the Fraser River has deposited finer-grained sands and silts within the floodplain, and other areas have accumulated eolian, lacustrine, bog, and stream sediments.

In the Miracle Valley, patches of peaty sediments have been mapped near the high point of the Valley, and colluvial slope deposits, including landslide fan gravels and rubble, blanket the toe of the south-facing bench (all Salish deposits). These are underlain by a thick sequence of clays and stony silt (Fort Langley Formation) and sandy till (Sumas Drift), which in turn is underlain by glaciofluvial sands and gravels (likely Vashon Drift). The lower-lying Hatzic Valley which borders the Miracle Valley is filled with more recent sands and silts (Fraser River).

The east wall of the Valley is blanketed by thick deposits of alluvial sediments reworked by numerous, mostly ephemeral watercourses (Fig. 3). Vast areas are covered by younger deciduous forest, possibly owing to the instability of these slopes. An extensive alluvial fan abuts the northeast end of the Valley as far as Cascade Creek. On the southeast end of the Valley, the Lagace Creek alluvial fan spills into the northeast corner of the Hatzic Valley.

2.4 SURFACE WATER

Several watercourses drain the east wall of the Valley (Fig. 3). On the north end of the Valley, they report to Cascade Creek (Photo 1), which in turn empties into Stave Lake. In the middle of the Valley, they report to Allan Lake (e.g., MacNab Creek) or to Lagace Creek directly. Some creek channels (e.g., Pattison Creek) show signs of extreme flows and channel erosion during periods of high rainfall/snowmelt (Photo 2). Given the coarse nature of their bed materials, these creeks may lose significant amounts of water to the subsurface upon reaching the valley floor. In late summer, many creeks are dry, and the water level in Allan Lake drops considerably (Photo 3).

Lagace Creek crosses the south end of the Valley (Photo 4) and then picks up flow from several tributaries on the west side, namely Belcharton Creek, Durieu Creek, Oru Creek, and Seux Brook (Photo 4). These creeks are relatively low grade and are interpreted to be largely spring-fed, as flow is relatively constant throughout the year. They are also incised in steep-sided ravines that cut into the topographic bench.

With the exception of Bouchier Creek at the south end and Marino Creek at the north end, there are no significant watercourses draining the west wall of the Valley. This wall is largely outcropping bedrock overlain by a veneer of colluvium and glacial drift. Marino Creek is sourced from spring-fed marshy areas at the foot of the rock quarry, and flows northward to Stave Lake.

A surface water divide exists at the high point in the Valley along Hartley Road. Runoff on the north side of Hartley Road flows towards Stave Lake and runoff on the south side reports to Lagace Creek. A subtle east-west surface water flow divide follows Burns Road. In this part of the Valley, the ground is soggy and poorly drained, owing to fine-textured, clayey soils. Boggy conditions also exist in the low-lying areas at the north end of the Valley (Photo 6).

Recent correspondence with BC Hydro confirms that Stave Lake water levels fluctuate between 75 and 82 m-asl over the year in response to flows released at the Stave Falls dam. This range in lake elevations has not changed since the beginning of the shared period of record (1983).

2.5 MIRACLE VALLEY AQUIFER

The glaciofluvial sands and gravels underlying the Fort Langley clays comprise a deep-confined aquifer known as the Miracle Valley Aquifer (the Aquifer). It has been mapped to cover an area of about 10 km², although there are some uncertainties regarding its northern boundary (Fig. 2).

The Aquifer is likely recharged by infiltrating surface runoff from Cascade Creek and ephemeral creeks draining the east wall of the Valley, and by slow infiltration of direct precipitation through the overlying aquitard. Several shallow, perched aquifers have been encountered in small pockets of sandy material within the clay, but these are considered to have limited supply potential owing to their small size and isolation from surface sources of recharge.

Numerous domestic water supply wells have been drilled into the Aquifer, and those that have been registered in the BC MOE's water well database are shown on Fig. 3. The driller estimated yields of these wells are generally greater than 1.5 L/s, which suggests a productive aquifer since most wells are constructed with short screens (less than 2m) extending a short distance (<5m) into the Aquifer. Wells of interest near the south end of the Valley include a well with an estimated yield of 47 L/s, and an artesian well that had at one time flowed at a rate of 16 L/s.

The Aquifer discharges to several springs at the south end of the Valley where topographic bench intercepts the piezometric surface. The springs are indicated by number on Fig. 3 and include:

- A number of spring vents that supply water to the Miracle Springs Hatchery and neighbouring property at 12697 Stave Lake Road (Spring No. 1). These springs are commonly referred to as the Lehmann Springs and at an elevation of about 76 m-asl. The combined flow rate from the springs has been estimated to be about 135 L/s, and is sustained year round¹. Most of flow is currently licensed for domestic use, bottled water sales, and pond maintenance.
- A series of springs issuing from coarse sediments in the banks Durieu Creek near its headwaters (Spring No. 2). These have been referred to as the Gadlatis Springs, and are at an elevation of about 79 m-asl. Flow from these springs has been estimated to be on the order of 35 L/s¹. The property owner currently holds spring water diversion licenses for domestic use, irrigation, and pond maintenance.
- Seux Brook is also reported to be sourced from springs near the top of Seux Road (Spring No. 3). Three residents on Seux Road hold licenses to divert spring water for domestic use. A small hatchery at the 35745 Durieu Road diverts flows from the Brook through fish-rearing tanks. Brook flows are highest in November and decrease to about 40 L/s in August².

Based on the above, Oru, Seux, Belcharton, and possibly Lagace Creek are interpreted to be largely groundwater-fed, especially along their lower reaches. The toe of the topographic

¹ Conversations with local residents during site visit by Allan Dakin of Piteau on September 13, 1994.

² Conversation with hatchery owner during site visit by Kathy Tixier or Piteau on December 6, 2011.

bench is blanketed by coarse landslide debris, which would provide little resistance to groundwater discharge.

These springs are distinct from springs discharging from fractured bedrock at west margin of the Valley. Fractures and joint sets have been observed in the west wall, which could convey groundwater flow from west to east. One such spring is located at 12699 Stave Lake Road at an elevation of 92 m-asl (above Belcharton Creek near the Miracle Valley Trout Hatchery), and is referred to as the Conroy Spring (Spring No. 4). The other is located on the Marino Creek bed on the north side of the rock quarry above 200 m-asl (Spring No. 5).

2.6 GROUNDWATER QUALITY

Laboratory analyses of a water sample previously collected from the Lehmann springs at the Miracle Valley Trout Hatchery indicate a relatively low degree of mineralization (TDS ~ 60 mg/L). This is unusually low for water originating from a deep confined aquifer, and suggests a relatively low residence time for groundwater in the Aquifer.

Groundwater samples were collected from domestic wells throughout the Valley as part of a Masters' research program (Magwood, 2004). Of the samples collected, approximately 10% had iron and manganese concentrations exceeding aesthetic guidelines of 0.3mg/L and 0.05 mg/L, respectively. Magwood (2004) also noted similarities in water chemistry between groundwater from the Aquifer and the adjacent Hatzic Prairie Aquifer to the south.

A search of the BC MOE Site Registry was conducted to locate potential groundwater pollution hazards in the Valley. The only notable result was a Notice of Independent Remediation Completion submitted for 14042-14100 Stave Lake Road on July 21, 2011. The notice states that a small volume (<700 m³) of soils potentially impacted by petroleum hydrocarbons were excavated in the vicinity of previously-existing underground fuel storage tanks and disposed of off-site. These soils are considered to impart a low level of risk to groundwater quality owing to their small volume and the fact that any downward migration of contaminants would be impeded by the confining clay layer. The risks associated with current land uses in the Valley are also judged to be low, given the minimal pesticide/herbicide use associated with the main agricultural crop (hay), and the low the density of ground disposal of sewage effluent.

3. SUMMARY OF INVESTIGATIONS

3.1 TEST WELL LOCATION SELECTION

Test well sites were selected at the north and south ends of the Valley to enable aquifer pump testing for evaluation of the potential for constructing future large-capacity production wells. Well locations were selected based on the following criteria:

- Not on private property or on Crown Lands;
- Accessible to truck-mounted well drilling and testing equipment;
- Clear of underground and overhead utilities;
- Near the centre of the Aquifer; and
- Where flowing artesian conditions unlikely to occur.

As over 90% of the lands overlying the Aquifer are privately owned, permits were obtained to locate test wells on Ministry of Transportation and Infrastructure (MOTI) right-of-way. A surveyor was retained to ground-truth cadastral boundaries, and utilities were contacted to verify underground service lines. On-site meetings were also conducted with drilling and site preparation contractors to mark out areas for drill pad construction and vehicular access.

3.2 DRILLING AND TEST WELL CONSTRUCTION

Test wells TW11-1 and 12-1 were drilled using a dual-mode air-rotary drilling rig operated by A&H Well Drilling Ltd. of Chilliwack, BC. The test wells were drilled and cased at a diameter of 200mm (8"), which can accommodate a submersible pump capable of pumping up to 40 L/s.

Logs with lithological information and well construction information are included with Appendices A and B along with results of grain size analyses conducted on formation samples collected from the screen completion zones. A more detailed account of formation conditions encountered and the design of each well are provided in the following sections.

3.2.1 TW11-1

Test well TW11-1 was drilled on the MOTI right-of-way at the south end of Burns Road (Photo 7). A small area adjacent to the road was cleared of vegetation and a gravel drill pad was constructed to facilitate access for the drilling rig and support vehicle. Drilling of the test well began on December 5, 2011 and was completed on December 13, 2011 (Photo 8).

Unconsolidated sediments were encountered from ground surface to a maximum drilled depth of 77.7m. These consisted of sandy overburden to 4.6m, clay to 27.4m, and sand and gravel to 77.7m. The ratio of sand to gravel varied across this latter interval, but was predominantly sand below 63m. At this depth, sediment samples changed colour from a brownish-grey to grey, and the drilling discharge water changed colour from a rusty brown to grey. Drilling was stopped at 77.7m since sediments appeared to be less permeable with depth (fining downward), and since the productivity of the overlying 50m of aquifer sediments was considered more than adequate to ultimately supply groundwater at a rate of 210 L/s.

A 4.8m long section of stainless-steel telescopic well screen with a slot size of 2.03mm (0.080") was installed in the test well casing and exposed to aquifer sediments between 54.4 and 59.2m. The screen was developed by airlift pumping using compressed air for approximately ten hours, when the rate of sand migration had diminished to acceptable levels. The static water level observed on December 14, 2011, following development of the well was 25.3m below ground level.

3.2.2 TW12-1

Test well TW12-1 was drilled on the MOTI right-of way which extends eastward from the intersection of Rodela and Stave Lake roads (Photo 9). Permission to access the drill site via roadways on the adjacent property to the north was obtained by the landowner. Well construction began on January 3, 2012, and was completed on January 11, 2012 (Photo 10).

Unconsolidated sediments were encountered from ground surface to a maximum drilled depth of 76.8m. Overburden consisting of silty sand and sandy clay were encountered to a depth of 6.1m, followed by clay to 38.1m. Below the clay, a silty sand transitioned to coarse sand and gravel at 41.1m. This unit extended to the total depth drilled of 76.8m, with some variation in the proportions of sand to gravel. A distinct colour change in the sediments from brownish-grey to grey, and in the drilling discharge water from rusty brown to grey, was observed at about 69m (Photo 11). Drilling was stopped at 76.8m since the productivity of overlying 37m of sand and gravel was judged to be sufficient to meet the targeted extraction rate of 210 L/s.

A 3.1m long section of stainless-steel telescopic well screen with slot size of 2.03mm (0.080") was installed in the test well casing and exposed to aquifer sediments between 72.6 and 75.7m. The screen was developed by airlift pumping for approximately six hours. The static water level observed on January 17 was 34.6m below ground level

3.3 AQUIFER PUMP TESTING

Precision Service and Pumps Ltd. (Precision) were retained to conduct aquifer pumping tests with TW11-1 and TW11-12 using temporarily installed submersible pumps powered by a diesel generator (Photo 12). In each case, a brief (two-hour) variable-rate pumping test was completed, followed by a 24-hour constant-rate test.

Water levels in the pumped wells were monitored using graduated electric tapes and a selflogging pressure transducer. Groundwater levels in nearby domestic wells were monitored using self-logging pressure transducers. Pumping rates were measured using an orifice plate device, and all pumped water was discharged onto plastic tarps or plywood on the ground before draining to nearby watercourses (Photo 13). Discharge from TW11-1 was released to a watercourse within 30m of the well, and ultimately reported to Oru Creek. Discharge from TW12-1 was released to a watercourse within 15m of the well. This flowed into a large pond on the adjacent property to the north and then into a tributary to Cascade Creek.

Summary tables of manual measurements collected during the pumping tests are included with Appendices C and D. Additional details on the aquifer pump testing program are given in the following sections:

3.3.1 Variable-Rate Testing

Variable-rate tests were conducted to evaluate the performance characteristics of the test wells, and to select a rate for the constant-rate tests. In each case, the test wells were pumped for 30 minutes at incrementally increasing rates while water level drawdown in the well was measured and recorded at frequent intervals.

3.3.2 Constant-Rate Testing

Constant-rate aquifer pumping tests were conducted after the wells had recovered from the variable-rate tests. This involved pumping the wells for 24 hours (1,440 minutes) at a constant rate of 34.7 L/s at TW11-1 and 33.1 L/s at TW12-1. Water level drawdown in the pumped wells was monitored at frequent intervals.

During the test with TW11-1, drawdown was also monitored in a domestic well located approximately 109m north of the pumped well at 12880 Burns Rd, henceforth referred to as OBS11-1. During the test with TW12-1, drawdown was also monitored in a well intended for fire suppression at 14042 Stave Lake Rd, approximately 120m from the pumped well (OBS12-1). Both observation wells are screened near the top of the confined aquifer at total depths of 40.8m and 53.3m, respectively. Driller's logs for these wells have also been included with Appendices A and B.

When pumping stopped, the recovery of water levels in the pumping wells and observation wells was monitored manually for at least two hours.

Analysis and interpretation of the constant-rate aquifer pumping test data is discussed in Section 4.2.

3.4 GROUNDWATER SAMPLE COLLECTION AND ANALYSES

Samples of drilling discharge water were collected during drilling of TW11-1 and TW12-1 from depths of 77.7 and 78.6m, respectively. The purpose of these samples was to investigate whether the colour change noted in the sediment / drilling discharge samples at 64 and 69m may be indicative of a change in groundwater chemistry and possibly an increase in iron and

manganese concentrations at depth. Samples of groundwater were also collected from OBS11-1 to sample groundwater quality near the top of the Aquifer, and as a good-will gesture to the owner for providing access to their well during aquifer pump testing.

Samples of groundwater were also collected from the discharge stream at the end of the constant-rate pumping tests with TW11-1 and TW12-1. All samples were submitted to ALS Environmental's Vancouver laboratory for analysis of basic potability parameters (including physical parameters, anions, nutrients, and metals), total coliform, E. coli bacteria, and radionuclides. Select samples were also tested for volatile organic compounds (VOCs), extractable petroleum hydrocarbons, and/or polycyclic aromatic hydrocarbons (PAHs). These additional analyses were requested to screen for the presence of petroleum hydrocarbons in the Aquifer, particularly at TW12-1, where recent remediation of suspect hydrocarbon-impacted soils had been reported on the adjacent property.

3.5 GROUNDWATER AND SURFACE WATER LEVEL MONITORING

On December 6, 2011, temporary creek gauging stations were established at Durieu and Oru creeks at their culverted crossings of Durieu Road (Photos 14, 15). The purpose of this monitoring was to obtain a qualitative baseline record of flows in these creeks for interpretation of groundwater-surface water relationships. Each station was equipped with a self-logging pressure transducer suspended inside a 2" diameter perforated pipe (stilling tube) that was secured to a length of rebar driven vertically into the creek bed.

No station was installed at the Seux Brook crossing since discharge rates are controlled by the nearby fish hatchery at 35745 Durieu Road. Furthermore, we observed that Seux Brook crosses Seux Road above Durieu Road (rather than crossing Durieu Road east of Seux Road as indicated on Figs. 2 and 3)³. Based on a close inspection of recent aerial imagery, we infer that Seux Brook flows eastward from Seux Road to Oru Creek just above Durieu Road, and empties into Oru Creek just above our gauging station. Hence, flows measured at this station are interpreted to be the sum of flows from Oru Creek and Seux Brook.

³ Observed during Piteau site visit on December 6, 2011.

Manual flow measurements were conducted at both stations on December 14, 2011 and February 1, 2012. Flow was calculated based on the wetted cross-sectional area of the culvert and average velocities measured using a hand-held current meter.

Piezometric heads in TW11-1 and TW12-1 have been monitored since completion of the aquifer pumping tests using self-logging pressure transducers installed in the wells.

4. ANALYSIS AND INTERPRETATION

4.1 HYDROSTRATIGRAPHY

Sediments encountered in TW11-1 and TW12-1 were consistent with conditions anticipated based on driller's logs for neighbouring domestic wells. The sediment profile can be divided into two major hydrostratigraphic units:

- 1. Clay and Till (Fort Langley Formation, Sumas Drift): grey, with some fine sand and gravel, and overlain by up to 5m of loamy sand (Salish Sediments). This unit acts as a confining layer to the underlying aquifer and is saturated over most of its thickness.
- 2. Sand and Gravel (probably Vashon Drift): brown to grey sandy gravel and gravelly sand, moderately graded, with trace silt. This unit hosts the Miracle Valley Aquifer, and appeared to be very permeable, based on the texture of recovered samples and observed airlift yields during drilling and development. The vertical thickness of this unit is greater than 50m at TW11-1 and 35m at TW12-1. The piezometric head elevation at these wells was approximately 3 to 4m above the contact with the overlying clay unit.

Hydrostratigraphic profiles through the aquifer along section lines A-A', B-B' and C-C' are presented on Figs. 4, 5, and 6.

4.2 AQUIFER PROPERTIES

4.2.1 Aquifer Parameters

Drawdown observed during the variable-rate tests with TW11-1 and TW12-1 are plotted against a logarithmic timescale on the upper portions of Figs. E-1 and F-1 included with Appendices E and F. At the end of each test, the drawdown in the pumped well was significantly less than the available drawdown above the pump intake; hence, the maximum possible pumping rates from the installed submersible pumps were chosen for the constant-rate tests.

Drawdown measurements in the pumped well during the constant-rate tests with TW11-1 and TW12-1 are plotted against a logarithmic timescale on Figs. E-2 and F-2. On the same plots, residual drawdown during the recovery interval (after the pump is shut off) is plotted versus the logarithm of the time ratio⁴. Drawdown response measured in the observation wells during the pumping and recovery intervals of the constant-rate tests are plotted versus the logarithm of time in the upper portions of Figs. E-3 and F-3. The lower portions of Figs. E-3 and F-3 present the drawdown response measured in the observation wells in log-log format.

Aquifer transmissivity (T) ⁵ and storativity (S)⁶ values determined from the pumping interval data using the Cooper-Jacob (1946) and Theis methods are summarized in Table I. T and S values calculated from the recovery interval data using the Theis Recovery (1935) method are also shown in Table I. Representative T values of 1.6×10^{-1} for TW11-1 and 1.3×10^{-1} m²/s for TW12-1 were obtained from the geometric mean of the T values given by each of these methods. Dividing the average T values by 10 times the screen lengths of the wells (as a rough approximation of aquifer thickness contributing to flow, in consideration of partial penetration effects) gives mean hydraulic conductivity (K) values of 3.3×10^{-3} and 4.3×10^{-3} m/s at TW11-1 and TW12-1, respectively.

These K values are comparable to the K values estimated from the grain size distributions of sediment samples collected near the screened intervals of the pumped wells (Figs. A-1, B-1). Using the Hazen (1911) method, these Ks are 1.1×10^{-3} and 5.6×10^{-3} m/s at TW11-1 and TW12-1, respectively.

Representative S values obtained by analysis of the drawdown and recovery trends at the observation wells are 1.9×10^{-3} at TW11-1 and 2.4×10^{-5} at TW12-1. Confined aquifers have relatively small storage coefficients (10^{-5} to 10^{-3}), since the stored water is derived from the compression of aquifer and expansion of water when the hydrostatic pressure (head) is reduced during pumping. Hence, these S values appear to be reasonable.

⁴ Time ratio (t/t') = time since start of test (t) over time since pumping stopped (t')

⁵ Transmissivity (T) = the rate that groundwater would flow through a vertical slice of aquifer one metre wide under a hydraulic gradient of one metre per horizontal metre. It is also equivalent to the hydraulic conductivity (K) multiplied by the aquifer thickness (b).

⁶ Storativity (S) = the volume of water an aquifer releases from storage per unit surface area of aquifer per unit drop in head.

The semi-log plot on Fig. F-3 indicates a departure from the straight-line drawdown trend at OBS12-1 approximately 700 minutes (11.7 hrs) into the constant-rate test with TW12-1. The cause of this deflection is unknown, but may indicate the interception of an impermeable boundary, such as the west bedrock wall of the Valley. As bedrock does not transmit water as readily as saturated sediments, drawdown would be expected to increase as shown. This departure from the straight-line trend was not apparent at the pumped well (Fig. F-2).

The semi-log plot on Fig. E-3 shows a near-perfect overlap between the drawdown and recovery trends at OBS11-1. This may indicate that groundwater flow upgradient from TW11-1 was adequate to meet the pumping demand and that there was no significant depletion of aquifer storage. There is a slight deflection away from the straight-line near the end of the pumping interval; however, it is uncertain whether this is due to a boundary effect or activation of a nearby well. There is no deflection from the straight-line fit in the drawdown trend at the pumped well (Fig E-2).

Possible boundary effects may be determined with greater certainty by longer duration tests at either location.

4.2.2 Possible Well Yields

The foregoing analysis of confirms that the Aquifer is highly permeable. Yields for larger diameter (12" or 16") production wells constructed near TW11-1 and TW12-1 can be roughly estimated using the formula:

$$Q = D_{max} \times S_{c}$$

Where Q is the maximum sustainable flow rate, D_{max} is the maximum allowable drawdown, and S_c is the specific capacity.

For this exercise, the production wells are assumed to be larger diameter wells screened across the same intervals as TW11-1 and TW12-1. The maximum allowable drawdown (D_{max}) was estimated by multiplying the difference between the estimated lowest seasonal aquifer water level and the top of the screened interval by a 70% safety factor. The

specific capacity (S_c) was estimated from the projected 100-day specific capacity (Figs. E-2, F-2). This is a conservative estimate of S_c, since wells of larger diameter (and screen length) would have more open area than the test wells. As shown in Table II, the maximum yields for a production well constructed near TW11-1 and TW11-2 are estimated to be on the order of 124 L/s and 360 L/s, respectively. As this is higher than what can generally be achieved using a single well, the desired yield of 210 L/s could potentially be achieved using two or more larger diameter wells (12" or 16") at either location.

Notwithstanding the above, for a groundwater extraction rate of this magnitude to be sustainable, it must:

- Be balanced by available groundwater recharge;
- Not cause environmental harm, especially to aquatic environments (e.g., surface water); nor
- Interfere with other well drawing water from the same aquifer, and connected aquifers to the extent that they are unable to meet existing demands.

These considerations are discussed further in Section 4.5.

4.3 AQUIFER CONCEPTUAL MODEL

4.3.1 Groundwater Flow direction

A hydrostratigraphic profile along the north-south section line A-A' is presented on Fig. 4. At about 92 m-asl, the piezometric surface elevation is highest in the middle portion of the Valley (between TW11-1 and TW12-1). South of TW11-1, the piezometric surface elevation drops rapidly as a result of spring discharge to Durieu and Oru creeks. North of TW12-1, piezometric levels slope gradually toward Stave Lake (max. elevation 82 m-asl).

Based on these findings, we interpret there to be north-south groundwater flow divide in the mid-portion of the Valley, roughly in parallel with the surface water divide. Groundwater flow north of the divide discharges to Stave Lake, Cascade Creek and Marino Creek, and groundwater flow south of the divide discharges primarily to Belcharton, Durieu, Oru creeks and Seux Brook. Inferred groundwater flow directions are indicated in plan view on Fig. 3.

Figures 5 and 6 present the hydrostratigraphic profile along east-west section lines B-B' and C-C'. On Section C-C', the piezometric surface elevation rises from west to east across the Valley, indicating a westward component of groundwater flow. The locations of major springs along the west and south margins of the Aquifer (at the end of the groundwater flow path) are consistent with this observation. The east walls of the Valley are blanketed with coarse alluvial deposits, which act as effective drains for rainfall runoff and snowmelt. This surface water flows to Lagace Creek, which has downcut a steeply incised ravine downstream of Allan Lake. The section indicates that the Lagace Creek invert elevation is below the relatively thin confining clay unit and is in direct contact with Aquifer sediments. Therefore, there is likely significant infiltration of surface water into the Aquifer, thus elevating piezometric water levels on this side of the Valley.

4.3.2 Aquifer Recharge

Most recharge to the Aquifer is inferred to originate from Lagace Creek and its tributaries on the east side of the Valley, and possibly from runoff along the bedrock walls on the west side of the Valley. This interpretation is based on the following rationale:

 Temporal changes in groundwater and surface water levels between from December 2011 to February 2012 (Fig. 7): Precipitation amounts and ambient atmospheric temperature are shown in the top portion of the figure, and the remaining graphs present time-series of water levels at the Durieu and Oru creek stations, Stave Falls Dam, and test wells TW11-1 and TW12-1.

As would be expected water levels in Durieu and Oru creeks respond rapidly to heavy precipitation events. A similar response of lesser magnitude is detected in Aquifer water levels at TW11-1. Since the Aquifer is overlain by a thick sequence of clay that would delay groundwater response to recharge events, the response observed is indicative of the Aquifer being recharged directly from surface water. Another possible line of evidence is the steadily rising groundwater levels observed at TW11-1 between the end of December and mid-January. This may be a result of gradually increasing water levels and in Allan Lake and overflows to Lagace Creek, since Allan Lake effectively acts as a storage basin for mountain runoff.

- Rudimentary water balance calculations: Average annual recharge by direct infiltration is estimated to be on the order of 140 L/s, assuming an average annual precipitation of 1,800mm and infiltration rate of 30% across the footprint of the portion of the Aquifer south of the groundwater flow divide. This amount is much smaller than the estimated rate groundwater discharge to Creeks draining the south end of the Aquifer (on the order of 450 L/s, annual average). The difference can only be made up by infiltrating surface runoff along the Valley margins. For example, if 40% of the average annual precipitation falling within the Lagace and Belcharton creek catchments (Fig. 2) outside the Aquifer footprint infiltrates the Aquifer, this would constitute another 280 L/s of recharge.
- The elimination of Stave Lake as a source of recharge to the Aquifer, based on recorded lake water elevations below that of Aquifer piezometric elevations.

4.3.3 Potential Climate Change Impacts

Climate change is expected to impact groundwater resources across BC, owing to changes in both temperature and precipitation trends. Since the 1950's, BC's climate has warmed significantly and precipitation has increased slightly, although there are significant variations from region to region (Walker and Sidneysmith, 2008). Climate change models predict that the Coastal region of BC will experience a reduced snowpack, with more precipitation falling as rain during the winter months. Higher rainfall will result more frequent and higher volume runoff events. Warming temperatures will cause the spring freshet to occur earlier in the year, which in turn could extend the duration of the summer low-flow period and exacerbate limited streamflows in the late summer/early fall.

Shifts in the timing and amount of precipitation and streamflows will affect the amount of water recharging the Aquifer. A major proportion of this recharge is expected to be from exfiltration of surface water from Lagace and other creeks on the east side of the Valley. This rate of "leakage" is expected to vary with the wetted surface area of the creek channel. Aquifer water levels are expected to be highest during periods of high surface flow and lowest during periods of low surface flow, with some time lag. Higher streamflows during the winter months may more quickly top up groundwater levels; however, decreased snow accumulations on the local mountains may significantly shorten this recharge period. Groundwater levels during the summer months will largely follow the rate depletion of this stored water due to pumping withdrawals and groundwater discharge to creeks at the south end of the Aquifer (Belcharton, Durieu, Oru, Seux). If this recession period is lengthened, aquifer water levels may reach lower than average levels in the late summer/early fall, thereby making less water available for withdrawal and fisheries habitat.

Future shifts in the incidence of extreme climate events, are more difficult to predict, and will vary season to season and region to region across BC. Extreme droughts and extreme high rainfall/runoff events are expected to be buffered somewhat by the capacity of the Aquifer to store water, and affect groundwater flow dynamics in the short term.

4.4 AQUIFER WATER QUALITY

Laboratory reports for groundwater chemical analyses with TW11-1 and TW12-1 are summarized in Table III. In general, groundwater sampled from both wells is moderately mineralized (TDS 40 to 80 mg/L) and near neutral in pH. Water chemistry is identified as the calcium-carbonate type, with minor sulphate.

Analyses results for groundwater samples collected during the constant-rate test with TW11-1 are presented in column 1 of Table III. Concentrations of all constituents tested were below maximum allowable concentrations (MACs) and aesthetic objectives (AOs) in the Guidelines for Canadian Drinking Water Quality (GCDWQ, Health Canada). It is noteworthy that total and dissolved iron and manganese concentrations were less than method detection limits.

A sample collected from OBS11-1, screened at a higher elevation than TW11-1, had total iron concentrations exceeding the AOs of 0.1 mg/L and 0.05 mg/L, respectively (column 3). A sample of formation water collected 18m below the screened interval of TW11-1 during drilling (column 4) had detectable concentrations of dissolved iron and manganese. Although the formation water samples cannot be relied upon to be as representative as well water samples, these results suggest that iron and manganese concentrations vary with depth in the Aquifer.

Analyses results for groundwater samples collected during the pumping test with TW12-1 are presented in column 2 of Table III. Concentrations of all constituents tested were below GCDWQ MACs and AOs with the exception of:

- Lead, whose total and dissolved concentrations (both 0.012 mg/L) slightly exceeded the MAC of 0.01 mg/L;
- Manganese, whose total and dissolved concentrations (both 0.08 mg/L) slightly exceeded the AO of 0.05 mg/L.

The province occasionally collects water samples at select groundwater and surface water quality monitoring stations as part of the BC Environmental Monitoring System (EMS). One such station is located 190m northeast of TW12-1, and is screened within the Aquifer at a higher elevation (driller's log in Appendix B). The maximum concentrations of constituents analyzed between December 1992 and September 1993 are tabulated in column 6 of Table III. These also indicate a slight exceedance with respect to total lead (0.014 mg/L), but no detectable dissolved or total manganese. A sample of formation water collected 3.3m below the screened interval of TW12-1 during drilling had a dissolved manganese concentration of 0.04 mg/L (column 5). While the reliability of the EMS data is not proven, they affirm lead concentrations may be a concern in this part of the Aquifer, and that manganese concentrations may be spatially variable. The elevated lead concentrations are most likely the result of dissolution of naturally occurring lead-bearing minerals in the aquifer matrix.

Lead is considered as a cumulative general poison that can affect the central nervous system. Fetuses, infants, children up to six years of age, and pregnant women (because of their fetuses) are the most susceptible to adverse health effects. Presence of any detectable lead in a municipal scale water supply is undesirable, and concentrations above the GCDWQ would need to be reduced to very low levels before the water could be considered suitable for consumption by the general population.

Manganese concentrations exceeding the AO guideline are common in groundwater. This element has a low toxicity, and concentrations are limited to avoid problems resulting from precipitation and staining.

The green sand filtration process can be used to remove both lead and manganese from water and achieve very low residual concentrations. With a design filter capacity of 2 L/s/m², a filter area of about 105 m² would be needed to treat a flow of 210 L/s. This is equivalent to 40 cylindrical filter vessels with a diameter of 1.8m (6'). Additional detailed testing and system design by a water treatment specialist will be needed to determine actual treatment requirements.

4.5 SAFE AQUIFER YIELD

The results of this investigation show that the maximum achievable rate of groundwater withdrawal from municipal production wells constructed into the Aquifer near TW11-1, TW12-1 (or points in between) is likely greater than the District of Mission's target of 210 L/s. However, for a new groundwater extraction project to be considered sustainable, it will be necessary to demonstrate that it will be balanced by available groundwater recharge, that it will not result in net environmental harm, particularly to aquatic environments (e.g., surface water), and that it will not unduly interfere with other operating wells drawing water from the same Aquifer.

There is only limited data available on water wells drawing from the Aquifer, and the surface water regime in the study area, and further study is thus necessary to support a defensible assessment of the impacts of this groundwater extraction project. In accordance with the British Columbia *Environmental Assessment Act*, the scope of assessment will need to be developed in conjunction with an environmental consultant with input from the Environmental Assessment Office.

Based on the results of this assessment, and Piteau's experience with other environmental assessments for groundwater extraction, it can be anticipated that extraction of up to 210 L/s of groundwater from one or more municipal wells drawing from the Aquifer in the vicinity of TW11-1 (Burns Road) may reduce flows in Seux Brook, Oru, Durieu, and Belcharton creeks,

which are fed with groundwater from the Aquifer. Furthermore, limited surface water infiltration along the margins of the Valley during the summer drought period may reduce recharge to the Aquifer during periods of peak groundwater extraction.

An overview assessment of stream, fish, and wildlife resources in the Miracle Valley by Scott Resource Services (SRS, Appendix H) indicates that water use in Belcharton Creek is oversubscribed compared to available flows, and the BC Fisheries Inventory Summary System (FISS) has described this water use and diversion as a constraint for fisheries production. Scott anticipates that Seux Brook and Oru and Durieu creeks have similar hydrological constraints, and concludes that obtaining environmental approvals for a project to withdraw additional water from the Aquifer will be difficult, and would only be granted following adequate:

- modelling to quantity the effect;
- biophysical assessment to determine existing baseline environmental resources; and
- mitigation or compensation to offset the quantified effect on the relevant species.

Socio-economic impacts to holders of water licenses on these watercourses also warrant some consideration. A recent search of the BC MOE on-line Water Resources Atlas⁷ indicates several licensed points of diversion on Belcharton, Durieu, and Oru creeks and Seux Brook, most of which are at the springs feeding their headwaters. Most licenses are for domestic purposes at relatively small diversion rates (<0.1 L/s). Greater diversion rates have been approved for maintenance of pond levels (47-142 L/s), some of which support hatchery operations. These may be more susceptible to impacts owing to their greater demand, while domestic license holders may only be impacted if their spring sources were to shift location or dry up altogether.

A preliminary estimate of the steady state drawdown in the piezometric level in the Aquifer resulting from pumping a municipal production wells at the location of TW11-1 at 210 L/s is shown on Fig. 4. This is based on the drawdown observed at OBS11-1 during the aquifer pumping test with TW11-1, projected to 100 days and pro-rated to 210 L/s. Well(s) screened near the top of the Aquifer within 110m of the wells would be expected to experience 1.5m or more of interwell drawdown interference. Based on the driller's log (Appendix A) and static

⁷ Available on-line at http://www.env.gov.bc.ca/wsd/data_searches/wrbc/

water level measurements taken in December 2011, we conservatively estimate there to be 4.8m of available drawdown in OBS11-1, of which less than 1m is expected to be consumed by the installed pump. Hence, it is unlikely that this well would experience a loss in capacity due to interwell drawdown interference. Excluding OBS11-1, there appear to be no registered wells within 110m of TW11-1, and interwell drawdown interference of less than 1.5m is not expected to impact the performance of wells located outside this radius. However, this will need to be verified.

Extraction of up to 210 L/s of groundwater from one or more municipal wells drawing from the Aquifer in the vicinity of TW12-1 (Stave Lake Road) is less likely to significantly affect flows in Seux Brook, Oru, Durieu, and Belcharton creeks, as this location is further away and on the other side of a flow divide. As groundwater flow in this area is toward Stave Lake; a reduction in seepage toward the lake resulting from pumping a new well (or wells) at 210 L/s is not likely to have any significant effects on the lake. However, flows in Marino Creek and a nearby tributary to Cascade Creek, which are both reported by SRS to be fish-bearing reaches could potentially be affected.

A preliminary estimate of the steady-state drawdown resulting from pumping municipal wells at the location of TW12-1 at 210 L/s is shown on Fig. 4. This is based on the drawdown observed at OBS12-1 during the aquifer pumping test with TW12-1, projected to 100 days and pro-rated to 210 L/s. Well(s) screened near the top of the Aquifer within 120m of the wells would be expected to experience 4.1m or more of interwell drawdown interference. At OBS12-1, we conservatively estimate there to be 9.3m of available drawdown, based on the driller's log (Appendix B) and static water level measurements taken in January 2012. Therefore, there is a low likelihood that this well would experience a loss in capacity due to well interference caused by extracting groundwater from municipal wells at 210 L/s. Wells greater than 300m from the well field would be expected to experience less than 1m of interwell drawdown interference, and are thus even less likely to be affected. Impacts to wells within 300m (the MOE wells database indicates eight) should be done on an individual basis in consideration of their construction and operational characteristics. If well performance were compromised, drilling deeper wells could be considered as a form of mitigation.

5. ADDITIONAL INVESTIGATIONS

5.1 GENERAL RECOMMENATIONS

Depending on which strategy is pursued, additional field investigations and review will be needed to investigate the feasibility of groundwater supply options, to support an environmental assessment. Specific objectives of the work should include providing data to enhance the understanding of aquifer characteristics, assess the relationships between surface water and groundwater, and to inventory operating wells drawing from the Aquifer. The scope of these investigations should be developed in conjunction with an environmental consultant, and possibly with input from the Environmental Assessment Office.

As a minimum, the following investigations are recommended:

- Completing a survey of existing wells and operating springs in the study area to enable assessment of the potential effects that groundwater supply development in the Aquifer may have on them. Through a combination of mail-out questionnaires, door-to-door survey, and/or telephone contacts, the locations, elevations, and characteristics of all wells in the study area should be recorded. At this time, water samples could also be collected from selected wells to confirm groundwater quality in the vicinity of proposed municipal production wells. This would be a prudent next step in the vicinity of TW12-1 to confirm that lead concentrations exceeding the GCDWQ MAC are pervasive in this part of the Aquifer.
- Establishing and monitoring a surface water monitoring network to quantify flows in watercourses draining the southern end of the Aquifer (Oru, Durieu, Belcharton, and Lagace creeks) and establish a seasonal baseline. This would form the basis for assessing potential effects resulting from further groundwater supply development in the Aquifer. Basic inorganic quality could also be monitored. If warranted, flows in other creeks potentially affected by a well field near TW12-1 (e.g., Marino Creek) could also be monitored.

- Continuation of long-term monitoring of piezometric levels in TW11-1 and TW12-1 to determine range of variation, lowest seasonal level, and to provide data needed to better understand sources of recharge.
- Construction and testing of one or more larger diameter (12" to 16") test production wells and conducting high-rate (75 to 100 L/s) aquifer pumping test(s) over a minimum duration of one week in early autumn, when surface water flows and water levels in the aquifer are lowest. The surface and groundwater monitoring systems described above would be used to measure changes resulting from the test(s). Where permission can be obtained, water levels in a subset of private wells screened in the Aquifer should also be monitored.
- Developing a numerical groundwater flow model to simulate baseline conditions in the Aquifer and the response to groundwater withdrawals in terms of piezometric drawdown, and changes in groundwater inflow to creeks. If needed, the model could also be used to predict well capture zones.
- Preparation of one or more reports to summarize the program and results of work completed during this phase (Phase 2). These would document work completed, provide results of analysis and interpretation, and recommend strategies for sustainable groundwater supply development.

5.2 SPECIFIC RECOMMENDATIONS

Given the potential for surface water impacts to limit the maximum quantity of groundwater available from the south end of the Aquifer (Burns Road area), the District has indicated a preference to examine the water supply prospects in the area between TW12-1 and Stave Lake. If there is sufficient capacity, they are also interested in the possibility of extracting a much higher amount (~1,000 L/s). To this end, determining the extent of the Aquifer, and degree of hydraulic connection with Stave Lake, is now of primary interest.

Specific recommendations aimed at providing additional information in this regard are as follows:

- If available, review logs for any geotechnical boreholes drilled along the SW-NE trending BC Hydro power line right-of-way that crosses the north end of the Miracle Valley to determine what materials may be present in this area.
- Examine land ownership and reconnoiter the lands to the north of the power line right-of-way to identify possible drilling sites as close as possible to the lake high water level, and access routes. To maximize the probability of encountering a thick sequence of aquifer sediments, preference should be given to locations near the centre of the valley.
- After obtaining a temporary access agreement, retain a drilling contractor to drill a test well at a suitable location. The depth of drilling would depend on the soil and groundwater conditions encountered, but a maximum depth of 120m (400') is anticipated. A test well diameter of 200mm (8") is recommended, as this will accommodate a submersible pump capable of pumping at up to 40 L/s from 120m. Soil samples collected during drilling would be analyzed to define grain size distribution, and this data would be used to select appropriate sizes for well screens to be installed in the test well.
- If appropriate, conduct an aquifer pumping test with the test well to facilitate collection of groundwater samples and monitoring of aquifer and well performance. This would involve pumping at rates up to approximately 40 L/s and monitoring water levels in the pumped well and any other wells in the area (e.g., TW12-1). In addition to monitoring physical parameters during pumping, groundwater samples for a complete suite of analyses should be collected.

If the results of the test well drilling and aquifer pump testing program indicate favourable results, additional work would be needed to further assess the feasibility of extracting up to 1,000 L/s, and to provide information that would be needed in support of an environmental assessment. The scope of these investigations would be generally similar to described in Section 5.1. Additionally, if conditions warrant, consideration could also be given to conducting non-invasive geophysical surveys along a line parallel to the lakeshore using seismic refraction to assess the depth to bedrock, and/or the transient electromagnetic (TEM) method to identify potential water-bearing

zones based on their electrical conductivity. Coupled with the results of a test well in this area, the survey information could potentially be useful for mapping of the most productive parts of the Aquifer and for targeting high-capacity water supply wells.

6. CONCLUSIONS

Piteau Associates Engineering Ltd. has been retained by the District of Mission to conduct hydrogeologic investigations at the Miracle Valley to explore the feasibility of supplying up to 210 L/s of quality groundwater for municipal water supply. Our investigations have included the construction of two 200mm (8") diameter test wells conducting aquifer pumping tests with these wells.

The Miracle Valley is underlain by a sand and gravel aquifer that is confined by a thick sequence of clay and sandy till. Spatial and temporal trends in piezometric water levels indicate that primary source of recharge to the Aquifer is likely exfiltration from watercourses along the east side of the valley. Groundwater flow is interpreted to diverge to the north and south at a flow divide at the high point of the Valley (Hartley Road), and a westward component of flow is inferred in the southern portion of the Aquifer. At the southern limit of the Aquifer, watercourses which steeply incise the confining clay unit are predominantly spring-fed.

Aquifer pumping test data indicate that Aquifer sediments are highly permeable, with a hydraulic conductivity estimated to be on the order of 10^{-3} m/s. Theoretical short-term yields for larger diameter pumping wells constructed at or near test well sites TW11-1 and TW12-1 could range between about 124 and 360 L/s, respectively. It therefore appears possible to extract groundwater at 210 L/s from two or more larger diameter (12" to 16") wells at either location.

Groundwater quality measured in water samples collected from TW11-1 is excellent, with the concentration of all constituents analyzed were within limits recommended in the Guidelines for Canadian Drinking Water Quality. However, analyses conducted on shallower and deeper groundwater samples indicate that iron concentrations could exceed aesthetic objectives at other depth horizons. Concentrations of manganese and lead in groundwater samples collected from TW12-1 slightly exceed acceptable limits. Elevated lead concentrations were also detected at a neighbouring well completed at a shallower depth in the Aquifer.

A long-term groundwater withdrawal of 210 L/s in the vicinity of TW11-1 is expected to reduce flows in creeks draining the south portion of the Aquifer (Belcharton, Oru, Durieu creeks, and Seux Brook). As fish habitat is considered to be already compromised along some reaches, obtaining

environmental approval could be challenging. In the vicinity of TW12-1, project withdrawals are not expected to impact these creeks, as groundwater flow is interpreted to be northward toward Stave Lake. However, Marino Creek and a small tributary to Cascade Creek may potentially be affected.

Groundwater withdrawals of 210 L/s in the vicinity of TW11-1 or TW12-1 is not expected to significantly affect the performance of water supply wells identified in the near vicinity. This should be verified on a well to well basis based on information regarding the well's depth and current use.

In accordance with the British Columbia *Environmental Assessment Act,* an environmental assessment will be required for groundwater supply development if the total amount of groundwater to be extracted will exceed 75 L/s. The scope of assessment should be determined by an environmental consultant, with input from the Environmental Assessment Office. This may include:

- Completing a survey of existing wells and operating springs in targeted areas to assess the potential impacts of the project on their supply capacity.
- Monitoring flows and possibly water quality in watercourses draining the southern end of the Aquifer (Oru, Durieu, Belcharton, and Lagace creeks) to establish a seasonal baseline for assessing potential impacts of the project on aquatic life. Monitoring of flows in other watercourses (e.g., Allan Lake and Marino Creek).
- Continuation of long-term monitoring of piezometric levels in TW11-1 and TW12-1 to improve the current conceptual model of groundwater flow dynamics and sources of recharge.
- Conducting extended, high-rate (75 to 100 L/s) aquifer pumping test(s) with larger diameter (12 to 16") test production wells to measure potential effects on creeks and neighbouring wells/springs.
- Developing a numerical groundwater flow model to estimate aquifer response to project withdrawals in terms of piezometric drawdown and changes in groundwater inflow to creeks, and predict well capture zones.

More specific recommendations have also been provided to examine the water supply prospects in the north end of the Aquifer between TW12-1 and Stave Lake to investigate the extent of the Aquifer, degree of hydraulic connection with Stave Lake, and ultimately to supply a much greater quantity of groundwater (1,000 L/s). These recommendations include reviewing any geotechnical drilling information available for the BC Hydro right-of-way, determining land ownership and reconnoitring the lands to the north of the power line right-of-way to identify possible drilling sites as close as possible to the lake high water level, drilling a test well and conduct aquifer pumping tests. If results are favourable, additional work would be needed to assess the safe Aquifer yield in this area in support of an environmental assessment. The scope of these investigations would be generally similar to investigations recommended in the foregoing.



7. LIMITATIONS

This investigation has been conducted using a standard of care consistent with that expected of scientific and engineering professionals undertaking similar work under similar conditions in BC. No warranty is expressed or implied.

This report is prepared for the sole use of the District of Mission. Any use, interpretation, or reliance on this information by any third party is at the sole risk of that party, and Piteau accepts no liability for such unauthorized use.

Respectfully submitted,

PITEAU ASSOCIATES ENGINEERING LTD.

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TABLES

TABLE I

SUMMARY OF AQUIFER PARAMETERS ESTIMATED FROM CONSTANT-RATE PUMPING TEST RESULTS

| Pumped Well | Date | Pumping Rate (L/s) | Monitoring Point | Radius from Pumped Well (m) | Analysis Method | Transmissivity, T (m²/s) | Aquifer Thickness, b (m) | Hydraulic Conductivity, K (m/s) | Storativity () | Plot |
|----------------|------|--------------------------|---------------------|-----------------------------------|---------------------|-----------------------------|--------------------------------|---------------------------------------|-------------------|------|
| | - | | TW11-1 | 0 | Cooper-Jacob | 6.6E-02 | | | - | E2 |
| 7 | 201 | | (pumped well) | | Theis Recovery | 2.3E-01 | | | - | E2 |
| 71 | Dec- | 34.7 | | | Cooper-Jacob | 1.3E-01 | | | 1.6E-03 | E3 |
| F 1 | 5- | | OBS11-1 | 120 | Theis Recovery | 3.8E-01 | | | 2.6E-03 | E3 |
| | ~ | | | | Theis | 1.3E-01 | | | 1.7E-03 | E3 |
| | | | | | Representative Mean | 1.6E-01 | 48 | 3.3E-03 | 1.9E-03 | |

| Pumped Well | Date | Pumping Rate (L/s) | | Radius from Pumped Well (m) | | Transmissivity, T (m²/s) | Aquifer Thickness, b (m) | Hydraulic Conductivity, K (m/s) | Storativity () | Plot |
|------------------|--------|--------------------------|---------------|-----------------------------------|---------------------|-----------------------------|--------------------------------|---------------------------------------|-------------------|------|
| | 2 | | TW12-1 | | Cooper-Jacob | 6.6E-02 | | | - | F2 |
| ž | 201 | | (pumped well) | 0 | Theis Recovery | 2.3E-01 | | | - | F2 |
| V12 [.] | Jan- | 33.1 | | 109 | Cooper-Jacob | 1.3E-01 | | | 6.6E-06 | F3 |
| F | , Z | | OBS12-1 | | Theis Recovery | 1.2E-01 | | | 2.9E-06 | F3 |
| | 1 | | | | Theis | 1.7E-01 | | | 7.5E-04 | F3 |
| | | | | | Representative Mean | 1.3E-01 | 31 | 4.3E-03 | 2.4E-05 | |

TABLE II

ESTIMATED PRODUCTION WELL YIELDS

| | | | South Aquifer TW11-1 | North Aquifer TW12-1 |
|---|--|-------|----------------------------|----------------------------|
| | PARAMETER | Unit | Value | Value |
| Α | Static water level | m-bgl | 25.3 | 35.6 |
| В | Depth to top of screened interval | m-bgl | 56.4 | 71.8 |
| С | Allowance for seasonal variance | m | 2.0 | 2.0 |
| D | Available drawdown = B - A - C | m | 29.1 | 34.2 |
| Е | Safety Factor (SF) | | 30% | 30% |
| F | Allowance for interwell drawdown interference from other wells | m | 0.0 | 0.0 |
| G | Allowable drawdown (D_{max}) = D x (1-E) - F | m | 20.4 | 23.9 |
| | | L/s | 34.7 | 33.1 |
| Н | Test rate used for analysis | lgpm | 458.0 | 436.9 |
| | | USgpm | 550.0 | 524.6 |
| 1 | Projected drawdown after 100 days pumping at rate specified in (H) | m | 5.70 | 2.20 |
| J | Projected specific capacity at 100 days = H/I (S _c) | L/s/m | 6.1 | 15.0 |
| | | L/s | 124.0 | 360.2 |
| к | Estimated 100 day safe pumping rate = $D_{max} * S_c$ | lgpm | 1,637 | 4,754 |
| | | USgpm | 1,966 | 5,709 |

Notes:

m-bgl = metres below ground level

A: Static water level measured on Dec 14, 2011

E: For a 30% Factor of Safety, Allowable drawdown = 70% of Available drawdown

Calculations assume not interwell drawdown interference

E.

TABLE III

SUMMARY OF WATER QUALITY ANALYTICAL RESULTS

| Column No.: | | | | 1 | 2 | 3 | 4 | 5 | 6 |
|-------------------------------------|--------------|----------|---|---|--|-----------------------------------|--|--|---|
| | | | | | | | | | |
| Sample ID: | | | | TW11-1 | TW12-1 | LANG RESIDENCE ⁴ | TW11-1, 77.7m | TW12-1, 75.6m | BC EMS WELL ⁵ |
| Sample Description: | Units | | rinking Water delines ^{1, 2, 3} | Well discharge at end of pumping test | Well discharge at end of pumping test | Observation well OBS11-1 water | Formation water collected at 255 ft during drilling of TW11-1 | Formation water collected at 258 ft during drilling of TW12-1 | EMS water quality monitoring well near TW12-1 |
| Date & Time Sampled: | | | | 16-Dec-11 12:00 | 18-Jan-12 14:30 | 14-Dec-11 11:45 | 8-Dec-11 12:00 | 6-Jan-12 15:00 | Dec 1992-Sep 1993 |
| Laboratory COC: | | | | L1096762 COFC | L1105481 COFC | L1096345 COFC | L1093778 COFC | L1102004-1 COFC | EMSR0300 |
| Laboratory ID: | | | | L1096762 | L1105481 | L1096345 | L1093778 | L1102004-1 | |
| | | MAC/IMAC | AO / OG | | | | | | |
| Physical Chemistry | | | | | | | | | |
| Hardness (as CaCO3) | mg/L | - | - | 55.6 | 46.3 | 23.8 | 75.7 | - | 20.0 |
| Colour, True | CU | - | ≤15 | <5.0 | <5.0 | <5.0 | - | - | <5 |
| Conductivity | uS/cm | - | - | 122 | 64.5 | 59 | - | - | 53 |
| Field Conductivity | uS/cm | - | - | 172 | - | 300 | - | - | 6.7 |
| pH | pH | - | 6.5-8.5 | 7.72 | 7.60 | 7.51 | - | - | - |
| Field pH | pH | - | 6.5-8.5 | 7.21 76 | - 41 | 6.92 44 | - | - | - |
| Total Dissolved Solids Turbidity | mg/L NTU | - 1 | ≤500 | <0.10 | 41 0.23 | 44 1.12 | | - | - |
| Field Temperature | °C | - | - | 6.3 | - | 7.4 | - | - | - |
| UV Absorbance (254 nm) | Abs/cm-1 | - | - | <0.0010 | - | 0.12 | - | - | - |
| | Abaron-1 | - | | -0.0010 | - | 0.12 | - | - | - |
| Alkalinity, Total (as CaCO3) | ma/l | - | - | 49.5 | 47.3 | 23 | - | - | 22.3 |
| Bicarbonate (HCO3) | mg/L mg/L | - | - | 49.5 | - 47.5 | - 23 | - | - | 22.3 |
| Carbonate (CO3) | mg/L | - | - | <2.0 | - | - | - | - | - |
| Hydroxide (OH) | mg/L | - | - | <2.0 | - | - | | - | - |
| | iiig/∟ | _ | | -2.0 | - | - | - | - | - |
| | | | | | | | | | |
| Bromide (Br) | mg/L | - | - | - | - | - | - | - | - |
| Chloride (Cl) | mg/L | - | ≤250 | 0.95 | 0.70 | 1 | - | - | 0.90 |
| Fluoride (F) | mg/L | 1.5 | - | 0.031 | 0.030 | <0.020 | - | - | <0.1 |
| Sulfate (SO4) | mg/L | - | ≤500 | 9.41 | 10.7 | 2 | - | - | 2.2 |
| Nitrate (as N) | mg/L | 10 | - | 0.204 | 0.0729 | 1 | - | - | - |
| Nitrite (as N) | mg/L | 3.2 | - | <0.0010 | <0.0010 | <0.0010 | - | - | - |
| Nitrate plus Nitrite (as N) | mg/L | - | - | - | - | - | - | - | - |
| Ammonia (as N) | mg/L | - | - | - | - | - | - | - | - |
| | | | | | | | | | |
| Sulphide (as S) | mg/L | - | ≤0.05 | <0.020 | <0.0020 | - | - | - | - |
| Bacteriological Tests | MDN//400/ | | | | | | | | |
| Coliform Bacteria - Total | MPN/100mL | 0 | - | <1 | <1 | - | - | - | - |
| E. coli | MPN/100mL | 0 | - | <1 | <1 | - | - | - | - |
| Aluminum (Al) | mg/L | _ | 0.1 / 0.2 | <0.010 | <0.010 | <0.010 | 191 | - | 0.060 |
| Antimony (Sb) | mg/L | 0.006 | - | <0.00050 | <0.00050 | <0.00050 | <0.010 | - | <0.015 |
| Artimoty (05) Arsenic (As) | mg/L | 0.000 | - | 0.00090 | 0.00167 | 0.00010 | 0.092 | - | - |
| Barium (Ba) | mg/L | 1 | - | < 0.020 | < 0.020 | < 0.020 | 1.27 | - | < 0.001 |
| Boron (B) | mg/L | 5 | - | <0.10 | <0.10 | <0.10 | <0.0050 | - | 0.043 |
| Cadmium (Cd) | mg/L | 0.005 | - | <0.00020 | <0.0020 | <0.00020 | <0.10 | - | < 0.002 |
| Chromium (Cr) | mg/L | 0.05 | - | <0.0020 | <0.0020 | <0.0020 | 0.0058 | - | < 0.002 |
| Copper (Cu) | mg/L | - | ≤1.0 | <0.0010 | 0.0033 | 0.0064 | 0.735 | - | 0.055 |
| Iron (Fe) | mg/L | - | ≤0.3 | <0.030 | <0.030 | <u>0.197</u> | <u>300</u> | - | 0.07 |
| Lead (Pb) | mg/L | 0.01 | - | <0.00050 | 0.0120 | <0.00050 | 0.125 | - | 0.014 |
| Manganese (Mn) | mg/L | | ≤ 0.05 | <0.0020 | <u>0.0798</u> | 0.0089 | 6.62 | - | <0.002 |
| Mercury (Hg) | mg/L | 0.001 | - | <0.00020 | <0.00020 | <0.00020 | 0.00046 | - | |
| Selenium (Se) | mg/L | 0.01 | - | <0.0010 | <0.0010 | <0.0010 | <0.020 | - | <0.005 |
| Uranium (U) | mg/L | 0.02 | - | <0.00010 | <0.00010 | <0.00010 | 0.0067 | - | |
| Zinc (Zn) | mg/L | - | ≤5.0 | <0.050 | <0.050 | <0.050 | 0.663 | - | 0.03 |

TABLE III

SUMMARY OF WATER QUALITY ANALYTICAL RESULTS

| Column No.: | | | | 1 | 2 | 3 | 4 | 5 | 6 |
|-----------------------------------|-------|--|-------------------------|---|--|--|---|---|---|
| | | | | | | | | | BC EMS WELL ⁵ |
| Sample ID: Sample Description: | Units | Canadian Drinking Water Quality Guidelines ^{1,2,3} | | TW11-1 Well discharge at end of pumping test | TW12-1 Well discharge at end of pumping test | LANG RESIDENCE ⁴ Observation well OBS11-1 water | TW11-1, 77.7m Formation water collected at 255 ft during drilling of TW11-1 | TW12-1, 75.6m Formation water collected at 258 ft during drilling of TW12-1 | EMS water quality monitoring well near TW12-1 |
| Date & Time Sampled: | | | | 16-Dec-11 12:00 | 18-Jan-12 14:30 | 14-Dec-11 11:45 | 8-Dec-11 12:00 | 6-Jan-12 15:00 | Dec 1992-Sep 1993 |
| Laboratory COC: | | | | L1096762 COFC | L1105481 COFC | L1096345 COFC | L1093778 COFC | L1102004-1 COFC | EMSR0300 |
| Laboratory ID: | | | | L1096762 | L1105481 | L1096345 | L1093778 | L1102004-1 | |
| | | MAC/IMAC | AO / OG | | | | | | |
| Calcium (Ca) | mg/L | - | - | 16.1 | 14.6 | 7.76 | 153 | - | 6.9 |
| Magnesium (Mg) | mg/L | - | - | 3.46 | 2.45 | 1.07 | 129 | - | 0.73 |
| Potassium (K) | mg/L | - | - | 0.83 | 0.87 | 0.38 | - | - | 0.6 |
| Sodium (Na) | mg/L | - | ≤200 | 2.5 | <2.0 | <2.0 | 19.7 | - | 2.76 |
| Dissolved Metals | 9 | | | | | | | | |
| Aluminum (Al) | mg/L | - | 0.1 / 0.2 | <0.010 | <0.010 | - | 0.174 | - | <0.02 |
| Aluminum (Al) Antimony (Sb) | mg/L | 0.006 | - | <0.00050 | <0.00050 | - | 0.00705 | - | <0.02 |
| Antihony (Sb) Arsenic (As) | mg/L | 0.000 | - | 0.00091 | 0.000 | - | 0.0051 | - | 0.04 |
| Barium (Ba) | U | 1 | - | <0.020 | <0.022 | - | 0.0031 | - | 0.002 |
| Banum (Ba) Boron (B) | mg/L | 5 | - | <0.020 | <0.020 | - | <0.10 | - | <0.002 |
| | mg/L | 0.005 | | <0.10 | | | <0.000050 | | |
| Cadmium (Cd) | mg/L | | - | | < 0.0002 | - | | - | < 0.002 |
| Chromium (Cr) | mg/L | 0.05 | - | <0.0020 | < 0.0020 | - | < 0.00050 | - | < 0.002 |
| Copper (Cu) | mg/L | - | ≤1.0 | < 0.0010 | 0.003 | - | 0.0010 | - | 0.054 |
| Iron (Fe) | mg/L | - | ≤0.3 | < 0.030 | <0.030 | - | 0.105 | <0.03 | 0.012 |
| Lead (Pb) | mg/L | 0.01 | - | <0.00050 | 0.012 | - | < 0.0010 | - | <0.001 |
| Manganese (Mn) | mg/L | - | ≤ 0.05 | <0.0020 | 0.080 | - | 0.039 | 0.0361 | <0.002 |
| Mercury (Hg) | mg/L | 0.001 | - | <0.00020 | <0.00020 | - | <0.00020 | - | |
| Selenium (Se) | mg/L | 0.01 | - | <0.0010 | <0.0010 | - | 0.0012 | - | <0.003 |
| Uranium (U) | mg/L | 0.02 | - | <0.00010 | <0.00010 | - | 0.00054 | - | |
| Zinc (Zn) | mg/L | - | ≤5.0 | <0.050 | <0.00010 | - | <0.0050 | - | 0.021 |
| | | | | | | | | | |
| Calcium (Ca) | mg/L | - | - | 16.5 | 14.600 | - | 23.0 | - | 6.78 |
| Magnesium (Mg) | mg/L | - | - | 3.50 | 2.380 | - | 4.41 | - | 0.75 |
| Potassium (K) | mg/L | - | - | 0.81 | 0.860 | - | - | - | <0.4 |
| Sodium (Na) | mg/L | - | ≤200 | 2.5 | <2.0 | - | 6.1 | - | 1.95 |
| Radionuclides | | | | | | | | | |
| Gross Alpha | Bq/L | <0.5 | - | - | < 0.05 | - | - | - | - |
| Gross Beta | Bg/L | <1 | - | - | 0.05 | - | - | - | - |
| Volatile Organic Compounds | | | | | | | - | | |
| Benzene | mg/L | 0.005 | - | <.000050 | <0.00050 | | | | 1 |
| Ethylbenzene | mg/L | - | - ≤0.0024 | <.000050 | <0.00050 | - | | - | - |
| Methyl t-butyl ether (MTBE) | mg/L | - | <u>≤0.0024</u> 0.015 | <.000050 | <0.00050 | - | - | - | 1 |
| Toluene | mg/L | - | -≤0.024 | <.000050 | <0.00050 | - | - | - | - |
| ortho-Xylene | mg/L | | | <.000050 | <0.00050 | | | | |
| meta- & para-Xylene | mg/L | - | - | <.000050 | <0.00050 | - | - | - | - |
| | | - | - | | | | - | - | - |
| Xylenees | mg/L | - | ≤ 0.3 | <.000075 | <0.00075 | - | - | - | - |
| Hydrocarbons | | | | | | | | | |
| EPH 10-19 | mg/L | - | - | <0.25 | <0.25 | - | - | - | - |
| EPH 19-32 | mg/L | - | - | <0.25 | <0.25 | - | - | - | - |
| LEPH | mg/L | - | - | - | <0.25 | - | - | - | - |
| HEPH | mg/L | - | - | - | <0.25 | - | - | - | - |
| Volatile Hydrocarbons (VH6-10) | mg/L | - | - | - | <0.10 | - | - | - | - |
| VPH (C6-C10) | mg/L | - | - | - | <0.10 | - | - | - | - |

TABLE III

SUMMARY OF WATER QUALITY ANALYTICAL RESULTS

| Column No.: | | | | 1 | 2 | 3 | 4 | 5 | 6 |
|----------------------------------|-------|--|---------|---|--|-----------------------------------|--|--|---|
| Sample ID: | | | | TW11-1 | TW12-1 | LANG RESIDENCE ⁴ | TW11-1, 77.7m | TW12-1, 75.6m | BC EMS WELL ⁵ |
| Sample Description: | Units | Canadian Drinking Water Quality Guidelines ^{1,2,3} | | Well discharge at end of pumping test | Well discharge at end of pumping test | Observation well OBS11-1 water | Formation water collected at 255 ft during drilling of TW11-1 | Formation water collected at 258 ft during drilling of TW12-1 | EMS water quality monitoring well near TW12-1 |
| Date & Time Sampled: | | | | 16-Dec-11 12:00 | 18-Jan-12 14:30 | 14-Dec-11 11:45 | 8-Dec-11 12:00 | 6-Jan-12 15:00 | Dec 1992-Sep 1993 |
| Laboratory COC: | | | | L1096762 COFC | L1105481 COFC | L1096345 COFC | L1093778 COFC | L1102004-1 COFC | EMSR0300 |
| Laboratory ID: | | | | L1096762 | L1105481 | L1096345 | L1093778 | L1102004-1 | |
| | | MAC/IMAC | AO / OG | | | | | | |
| Polycyclic Aromatic Hydrocarbons | | | | | | | | | |
| Acenaphthene | mg/L | - | - | - | < 0.000050 | - | - | - | - |
| Acenaphthylene | mg/L | - | - | - | <0.000050 | - | - | - | - |
| Acridine | mg/L | - | - | - | < 0.000050 | - | - | - | - |
| Anthracene | mg/L | - | - | - | <0.000050 | - | - | - | - |
| Benz(a)anthracene | mg/L | - | - | - | <0.000050 | - | - | - | - |
| Benzo(a)pyrene | mg/L | - | - | - | <0.000010 | - | - | - | - |
| Benzo(b)fluoranthene | mg/L | - | - | - | <0.000050 | - | - | - | - |
| Benzo(g,h,i)perylene | mg/L | - | - | - | <0.000050 | - | - | - | - |
| Benzo(k)fluoranthene | mg/L | - | - | - | <0.000050 | - | - | - | - |
| Chrysene | mg/L | - | - | - | <0.000050 | - | - | - | - |
| Dibenz(a,h)anthracene | mg/L | - | - | - | <0.000050 | - | - | - | - |
| Fluoranthene | mg/L | - | - | - | <0.000050 | - | - | - | - |
| Fluorene | mg/L | - | - | - | <0.000050 | - | - | - | - |
| Indeno(1,2,3-c,d)pyrene | mg/L | - | - | - | <0.000050 | - | - | - | - |
| Naphthalene | mg/L | - | - | - | <0.000050 | - | - | - | - |
| Phenanthrene | mg/L | - | - | - | <0.000050 | - | - | - | - |
| Pyrene | mg/L | - | - | - | <0.000050 | - | - | - | - |
| Quinoline | mg/L | - | - | - | < 0.000050 | - | - | - | - |

Notes:

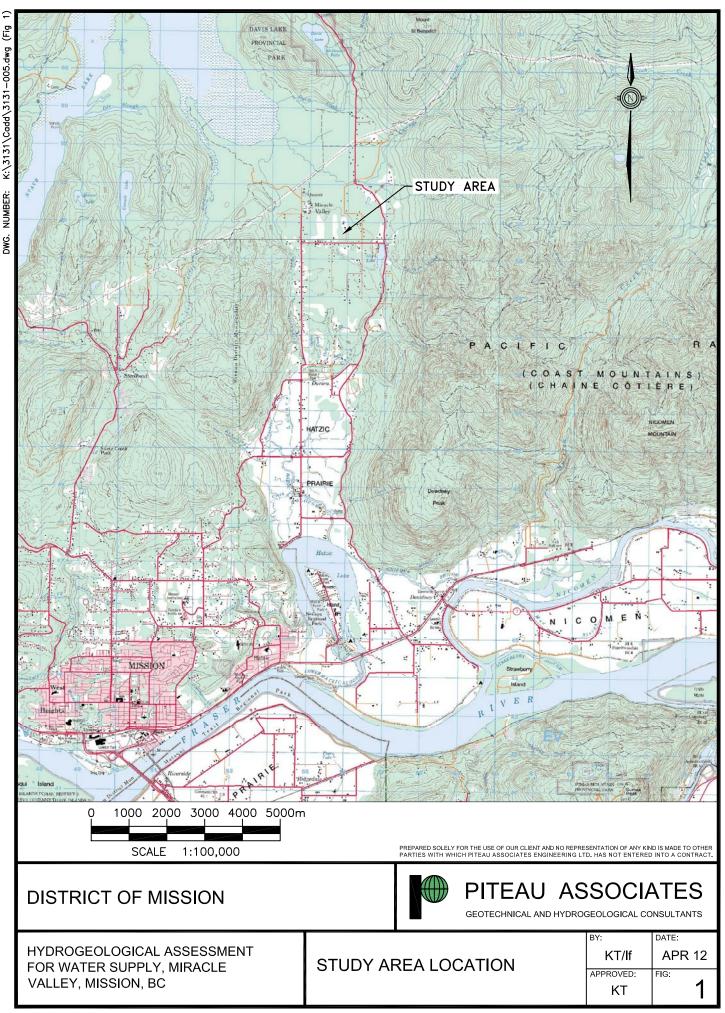
1. IMAC/MAC Interim and maximum allowable concentration; AO - Aesthetic objective; OG - Operational guidance value; Health Canada, May 2008.

2. AO is for sulphide as H₂S

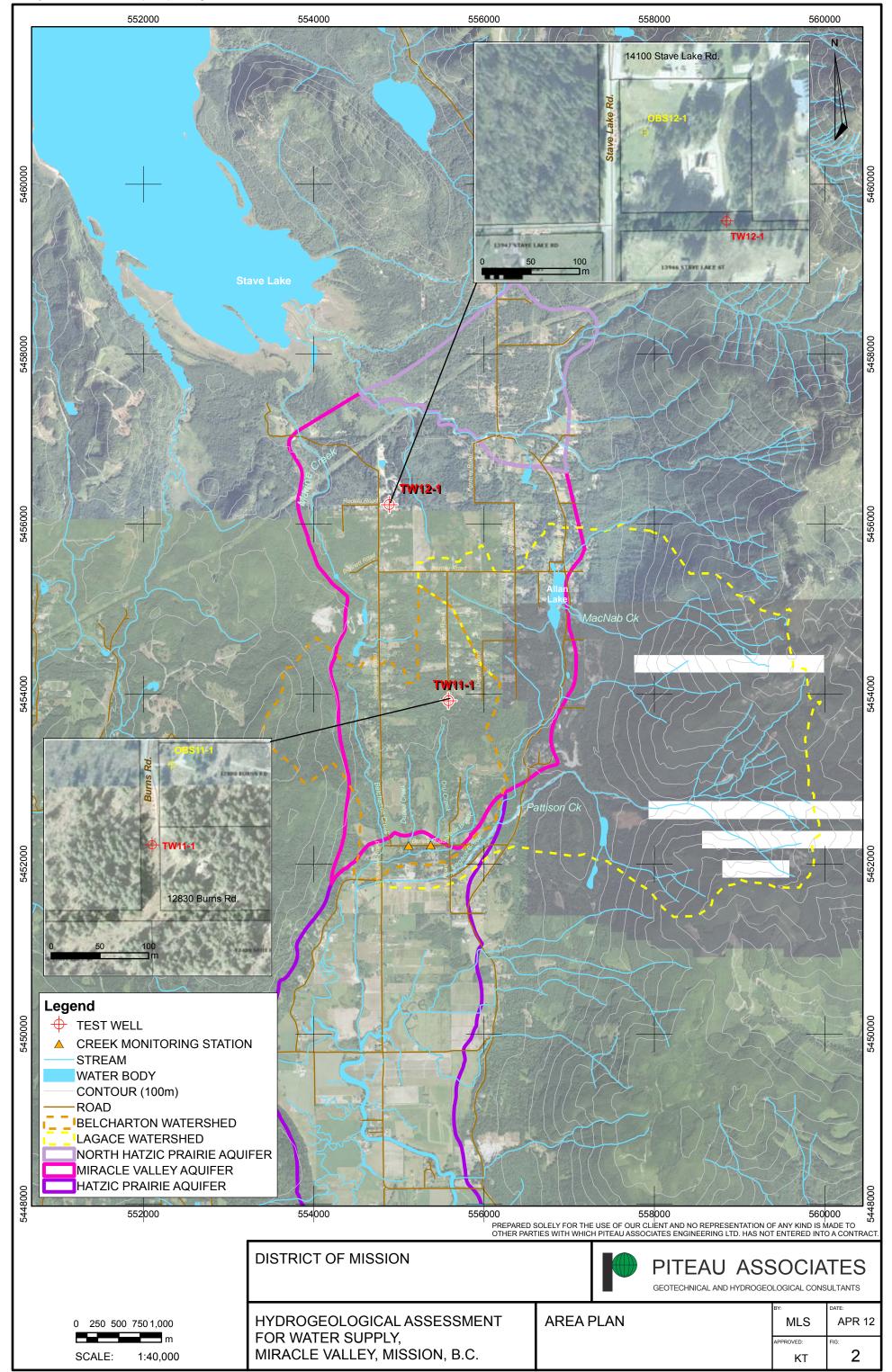
3. Bold and <u>underlined</u> cells denotes concentration exceeding IMAC/MAC or AO, respectively.

4. Sample obtained from domestic well at 12880 Burns Rd, used as an observation well during pumping test with TW11-1.

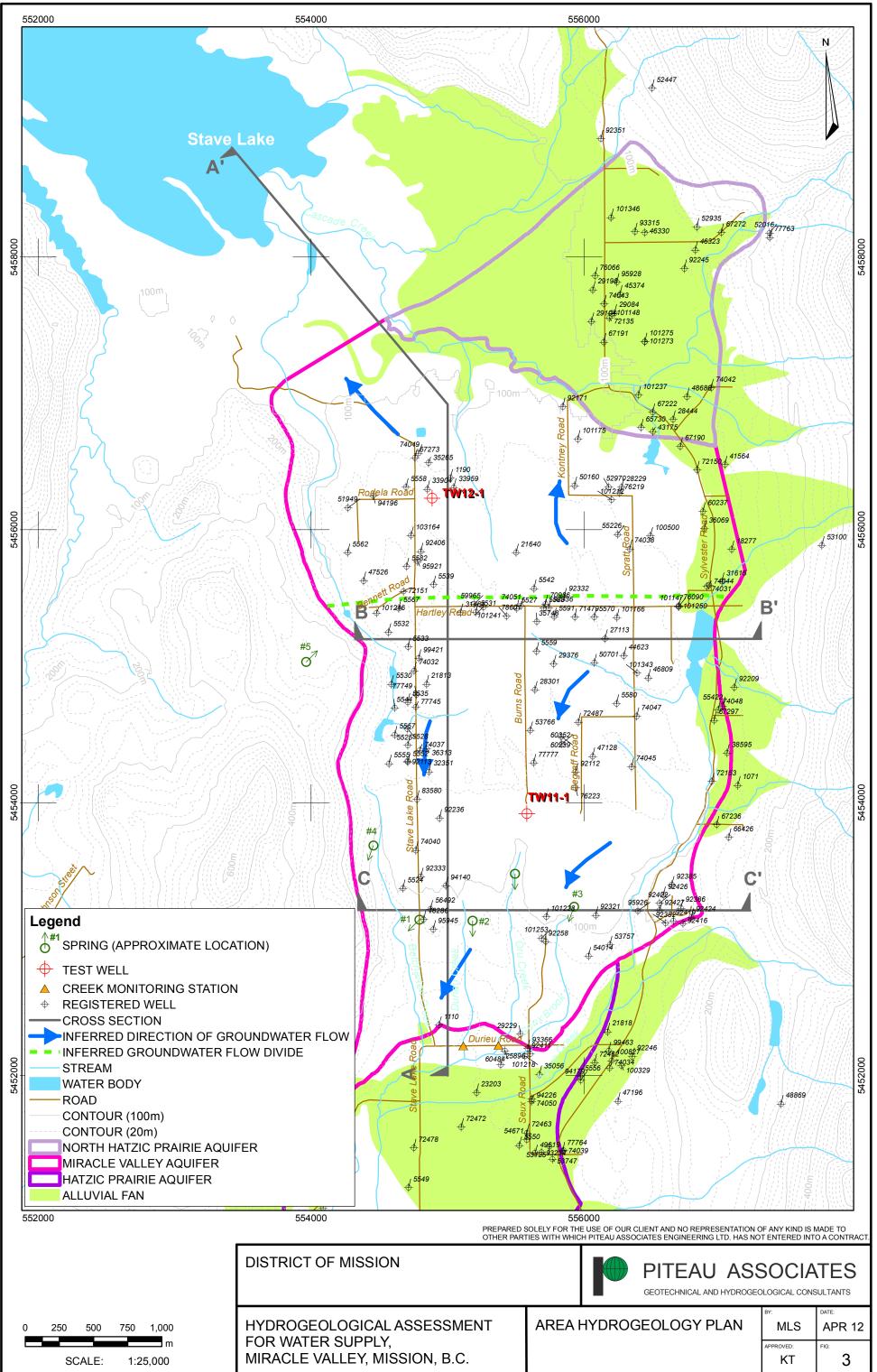
 BC Environmental Monitoring System historical statistics report for water well ID EMSR0300, located approx. 190m NE of TW12-1, 44m deep. Values given are historical maximum values measured between Dec 1992 and Sep 1993. **FIGURES**

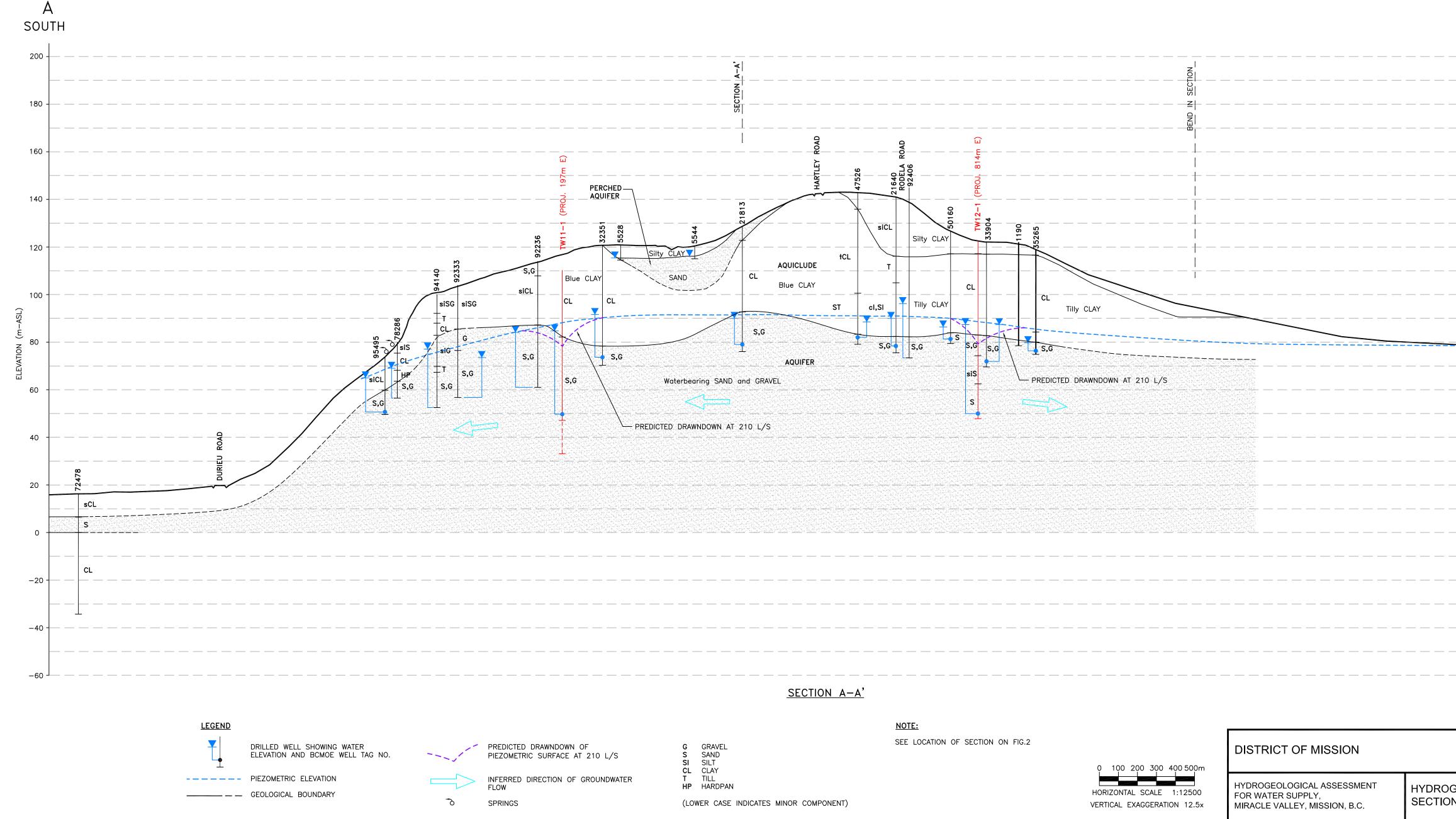


DWG. NUMBER: K:\3131\Cadd\3131-005.dwg (Fig

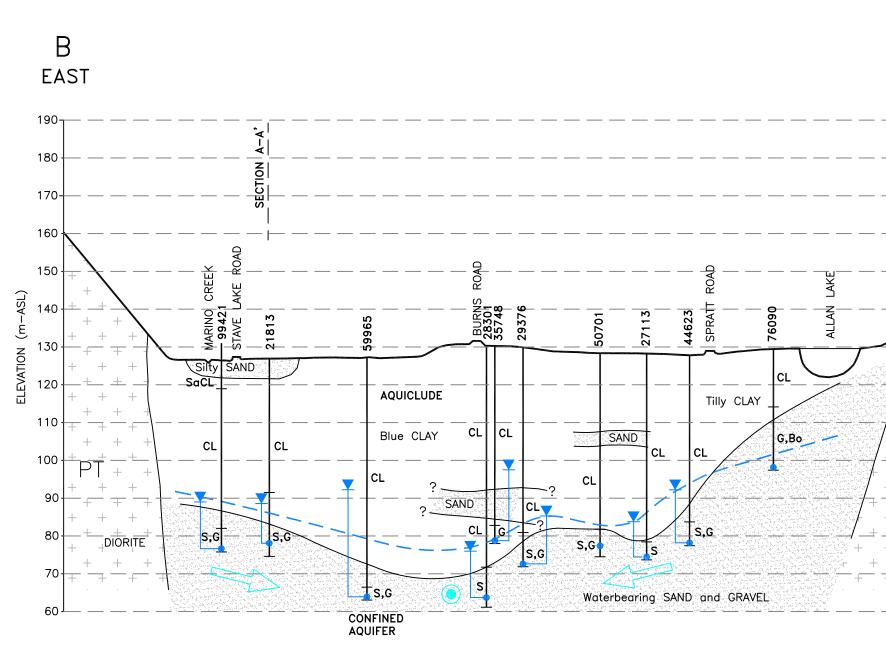


H:\Project\3131\ArcGIS\Arcmaps\Reports\Fig 3 - Area Plan With Work Details.mxd

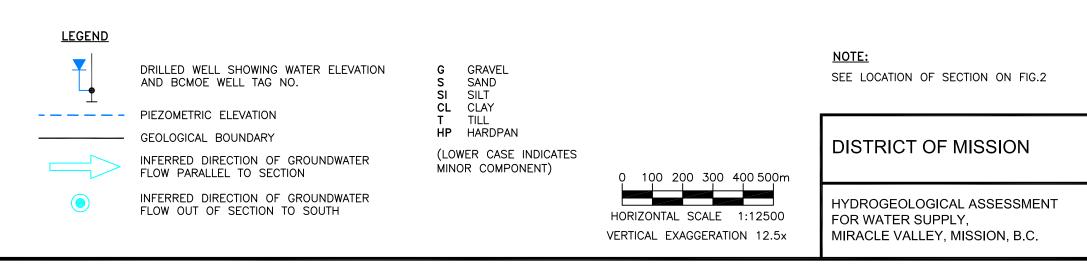




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| PF P <i>P</i> | REPARED SC ARTIES WIT | DLELY FC | H PITEAU | J ASSOC | ATES EN | GINEERIN | G LTD. | HAS NO | T ENTER | KIND IS MAD ED INTO A | CONTRACT. |
| PF P/ | REPARED SC | DLELY FC | OR THE US | SE OF OU J ASSOCI | R CLIENT / | AND NO RE | PRESE G LTD. | ENTATION HAS NO | OF ANY T ENTER | KIND IS MAD | E TO OTHER CONTRACT. |
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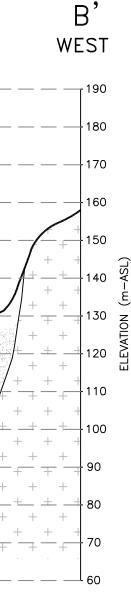
SECTION B-B'

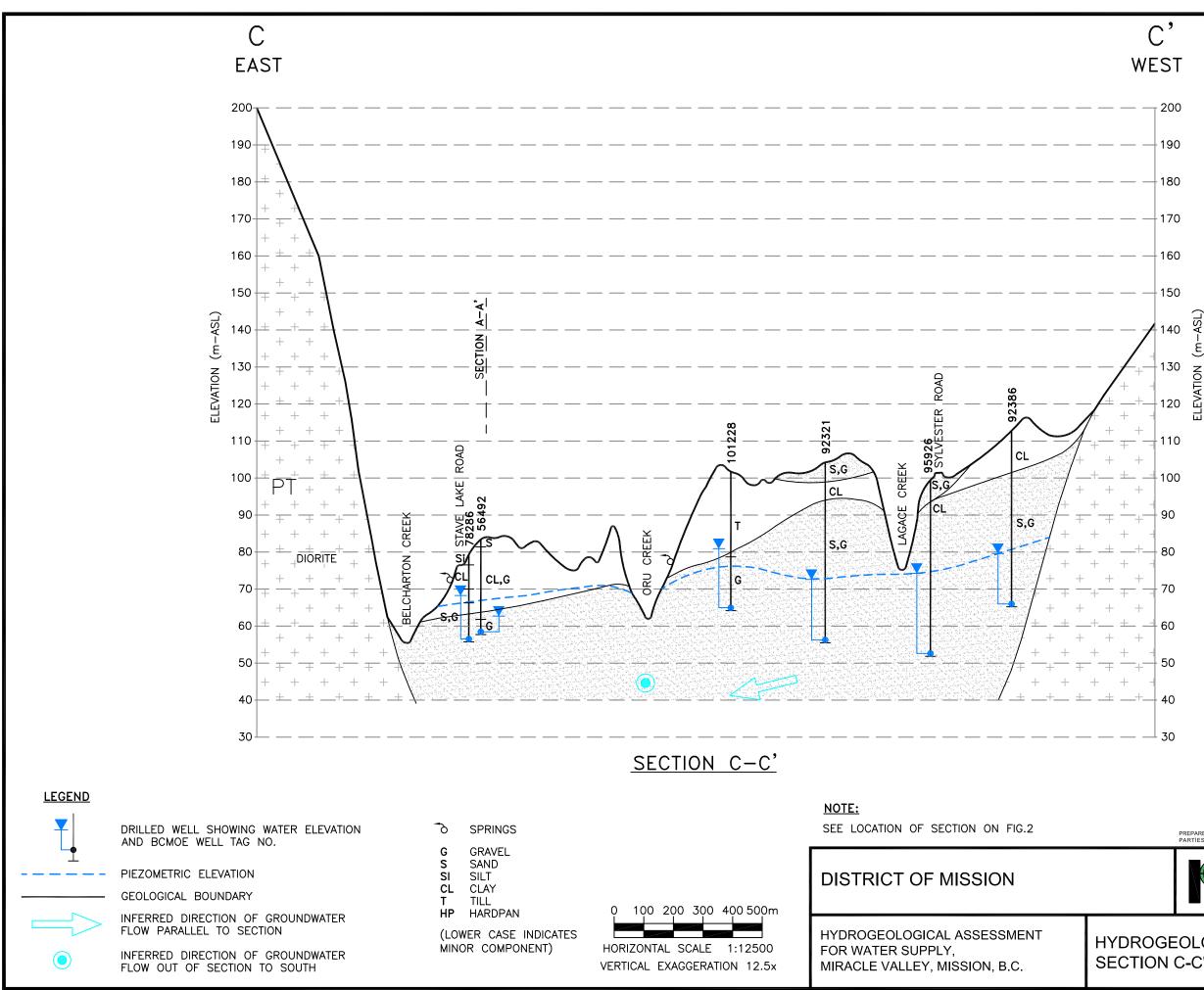


| GEOTECHNICAL AND HYDROG | GEOLOGICAL CC | NSULTANTS |
|-------------------------|---------------|-----------|
| | BY: | DATE: |
| HYDROGEOLOGICAL | DJT/sl | APR 12 |
| SECTION B-B' | APPROVED: | FIG: |
| | KT | 5 |

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PREPARED SOLELY FOR THE USE OF OUR CLIENT AND NO REPRESENTATION OF ANY KIND IS MADE TO OTHE PARTIES WITH WHICH PITEAU ASSOCIATES ENGINEERING LTD. HAS NOT ENTERED INTO A CONTRACT



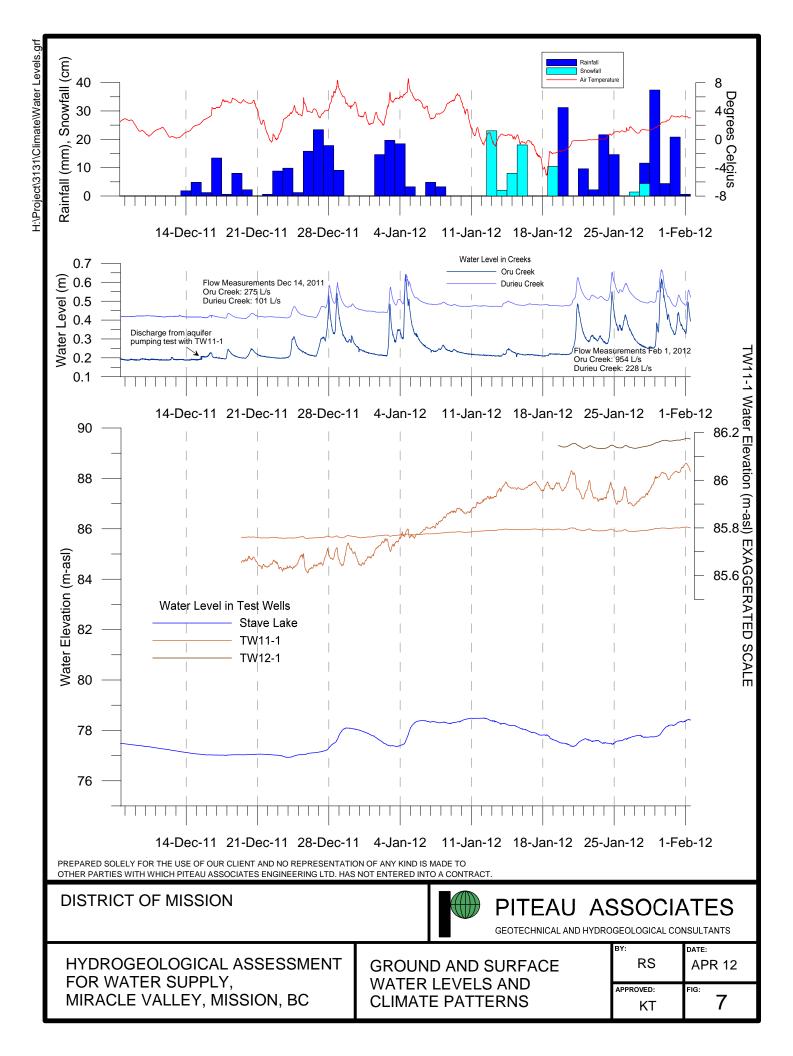


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| HYDROGE | EOLOGICA | ۸L | _{вү:} DJT/sl | DATE: APR 12 |
| SECTION | C-C' | | APPROVED: KT | FIG: 6 |

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PHOTOS



<u>Photo 1.</u> Cascade Creek near Kontney Road looking east, October 13, 2011.



Photo 2. Pattison Creek at Sylvester Road looking west, October 13, 2011.



<u>Photo 3.</u> Low water level in Allan Lake, October 13, 2011. Creek entering lake on north end is dry.



<u>Photo 4.</u> Lagace Creek at Farms Road looking east, October 13, 2011.



<u>Photo 5.</u> Oru Creek on north side of Durieu Road, December 6, 2011.



<u>Photo 6.</u> Marshy area along Kontney Road near Cascade Creek, October 13, 2011.



<u>Photo 7.</u> Location of test well TW11-1 at south terminus of Burns Road, February 1, 2012.



<u>Photo 8.</u> Drilling TW11-1 on December 6, 2011, with 20ft lengths of 8 inch well casing in foreground.



<u>Photo 9.</u> Location of test well TW12-1 adjacent to 14042 Stave Lake Road, February 1, 2012.



<u>Photo 10.</u> Drill rig and support truck at TW12-1, January 6, 2012.



<u>Photo 11.</u> Colour change in sediment samples from greyish-brown (210') to grey (234') at TW12-1, January 6, 2012.



<u>Photo 12.</u> Equipment configuration at well head during pumping tests with TW11-1, December 14, 2011.

PITEAU ASSOCIATES ENGINEERING LTD.



<u>Photo 13.</u> Orifice plate for measuring of well discharge rate during pumping tests with TW11-1, December 14, 2011.



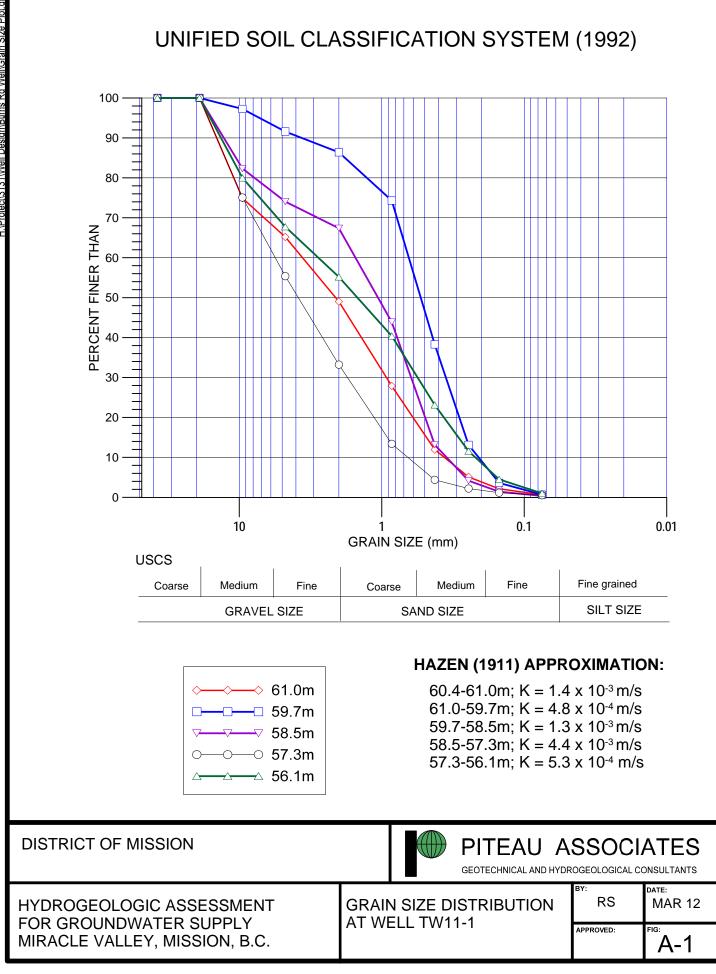
<u>Photo 14.</u> Durieu Creek gauging station at Durieu Road, December 6, 2011. Stilling tube is at outlet of culvert.





APPENDIX A

TW11-1 LOG AND GRAIN SIZE ANALYSES RESULTS PUMPING TEST OBSERVATION WELL LOG



| | | EAU ASSOCIATES | Те | st We | ell: TW11-1 | Page 1 of 2 |
|--|-----------------------------|--|-------------------|---------------------------|--|---------------------------|
| Location: Easting: Northing: Elevation | : Burn 555578 : 54539 | ns Rd, Miracle Valley BC 8 915 | Cli Pro Log | ent: D oject N gged | lled: Dec 5 - 13, 2011 istrict of Mission Number: 3131 By: R. Segovia e Diameter: 203 mm | |
| Depth Below Ground Surface | Elevation (maSL) | Lithologic Description | Depth (mbg) | Lithology | Remarks | Constructed Well |
| $\begin{array}{c} \text{ft} \text{m} \\ -5 \\ 0 \\ 10 \\ 15 \\ 10 \\ 15 \\ 10 \\ 15 \\ 10 \\ 11 \\ 10 \\ 11 \\ 11$ | | Ground Surface SAND WITH CLAY Brown medium sand with clay, trace gravel GRAVELLY CLAY Grey gravelly clay, some silt Wet below 5.3m CLAY Grey clay, some silt Serve clay, some silt Grey clay, some silt Sand CLAY Grey clay, some silt Sand clay, some gravel Sand clay, some gravel Brown medium to coarse sand and gravel, well graded | 4.6 | | Surface Completion: 0.91 m stick-up with locking cap Surface Seal: 305 mm surface casing installed to 6.1 m and removed during installation of bentonite seal | 11 25.3m bgl Dec 14, 2011 |
| Well Plate | ompa | 33361 ny: A&H Drilling l: Dual Rotary | 1 | <u>_000</u> | | |



PITEAU ASSOCIATES

GEOTECHNICAL AND HYDROGEOLOGICAL CONSULTANTS

Test Well: TW11-1 Project Number: 3131

Page 2 of 2

| Depth Below Ground Surface | | Lithologic Description | Depth (mbg) | Lithology | Remarks | Constructed Well |
|--|--|---|-------------|---------------|--|--------------------------------|
| 130 <u>-</u> 135 <u>-</u> | | GRAVEL Brown gravel with sand, well graded sub- rounded grains up to 1" | 41.5 | 10°0°0°0°0°0° | | |
| 140 145 155 155 165 165 170 175 175 180 185 185 185 185 185 185 185 185 | | SAND AND GRAVEL Brown medium to coarse sand and gravel, well graded sub-rounded grains up to 3/4" | | | 203 mm casing drilled to 77.7 m and pulled back to 53.9 m to expose screen Telescopic Stainless Steel Well Screen Assembly : Exposed from 53.9 m to 59.2 m K-Packer and Solid Riser: 53.6 to 54.4m | Riser → K-packer |
| 90 - 58 - 58 95 00 | | | | | Screen Interval: 54.4 to 59.2 m Slot Size: 2.03 mm | |
| 05 - 63 - 63 10 | | SAND Grey medium sand, poorly graded, some gravel | 62.8 | | | |
| 15 20 25 25 30 35 11 11 11 11 11 11 11 11 11 11 11 11 11 | | SAND Grey medium sand, poorly graded | | | | Mative bac |
| 40 73 45 | | | 77.7 | | | |

NEC-12-2011(WON), 13-28 FIELDODILLINGCONT

Field Drilling

(EAX)604 857 2267

TWII-1 OBS Well

P 001/001

25777-2747 Aquadiy 2 Revenilegeting "

et ha production

Aldergrove, BC V4W 2V1

Fax: (604)857-2267

WATER WELL RECORD

CONTRACTORS 'TD

| OWNER, ENGI, LEONARD | | | | DATE. NOVEMBER 29, 2005 | | | |
|-------------------------------|----------------|--------------|--|--------------------------------|-----------------------|--|--|
| ADDRESS: 5236 BOX 1 | | | | SITE ADDRESS: 12880 BURNS ROAD | | | |
| HEDLEY, BC VOX 186 | | | | MISSION, RC_ | | | |
| PHONE: 604-826-36 | 33 | | , | | | | |
| Date Begun: | NOVEMB | ER 23, 2005 | FROM | TO | WELL LOG DESCRIPTION | | |
| Date Completed: | | RR 28, 2005. | 0 | 20 | SAND AND CLAY- | | |
| Hole Diameter: . | 6 - | Inch. | 20 | 105 | CLAY WITH GRAVEL | | |
| Surface casing: | - 4 | _ | 105 | 135. | W.B. SAND AND GRAVEL | | |
| Diaz.8 | 18_ | Foot_ | 105 | 135. | W.B. SAIND AND GRAVEL | | |
| Drive shoe: | yes | | | · · | | | |
| MEASUREMENTS GROUND LEVEL: | FROM | | | | | | |
| Stick-Upt . | 3_ | Feet_ | ······································ | | | | |
| Bottom of Casing: | 129*6" | Feet | ļ | | | | |
| Hole Depth: | 134 | Feet | | | | | |
| Open Hole: From: | | Feet | | | | | |
| To: SCREENS: | | Feet | | | | | |
| Number of Screens: Slot Size: | 1 | | | | | | |
| Slot 30 | Slot | | | | | | |
| Slot | Slot | | | | | | |
| Screen Length: | 4 | Feet | | | | | |
| Ľ | 8 | Inch | | | | | |
| Top at: 127 Feet | | Inch | | | | | |
| Bottom At: 134 Feet | | Inch | | | | | |
| I. De chasses March | | | | | | | |
| K. Packer: YES | Riser; | 2FT | | | | | |
| B. Bottom: YES | | | | | | | |
| WELL | | | | | | | |
| COMPLETION: | | | | | | | |
| Rate: | 20 | GPM | | * • • * * | | | |
| Pump_Setting: | 125 | Foet | | | | | |
| Static Water Level: 🤇 | > | Feet | | | · · · · · | | |
| Develop: | 1 | Hours | L | • | | | |

SITE LEGAL DESCRIPTION:

WELL LD. 11545

NAME: ENGH, LEONARD JOB NUMBER: 2923-5

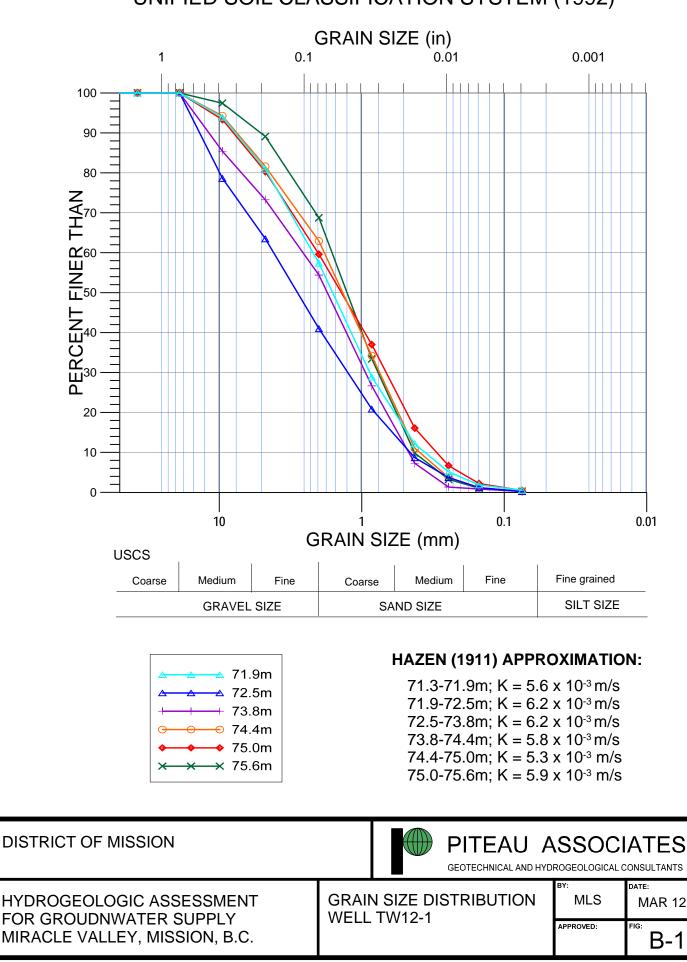
Rig No: 5 Rotary: YES Cable: Driller: LARRY FIELD Helper: STEVE JANZEN

的医地震管肝带

APPENDIX B

TW012-1 LOG AND GRAIN SIZE ANALYSES RESULTS PUMPING TEST OBSERVATION WELL LOG PROVINCIAL EMS WELL LOG

UNIFIED SOIL CLASSIFICATION SYSTEM (1992)



| | | EAU ASSOCIATES | Te | st we | II: TW12-1 | Page 1 of 2 |
|---|------------------|--|-----------------------------|---|---|--------------------------|
| GEOTECHNICAL AND HYDROGEOLOGICAL CONSULTANTS Location: Stave Lk Rd, Miracle Valley BC Easting: 554897 Northing: 5456251 Elevation: 129 maSL | | Clie Pro Log | ent: D oject N gged I | led: JAN 3-11 2012 istrict of Mission lumber: 3131 By: K. Tixier e Diameter: 203 mm | | |
| Depth Below Ground Surface | Elevation (maSL) | Lithologic Description | Depth (mbg) | Lithology | Remarks | Constructed Well |
| $\begin{array}{c} \cdot 3 \\ -3 \\ 2 \\ 17 \\ 12 \\ 17 \\ 12 \\ 17 \\ 12 \\ 17 \\ 12 \\ 17 \\ 11 \\ 17 \\ 11 \\ 17 \\ 11 \\ 11$ | | Ground Surface SILTY SAND Brown silty fine to coarse sand, some clay, some subangular gravel SANDY CLAY Light brown sandy clay, some subangular gravel CLAY Grey clay with fine sand, trace rounded gravel, increasing fine sand and gravel content with depth | 3.0 | | Surface Completion: 1.0m stick-up with locking cap Surface Seal: 305 mm surface casing installed to 6.1 m and removed during installation of bentonite seal. | 34.6 m bgl Jan. 17, 2012 |
| 112 | | 33369 ny: A&H Drilling | 38.1 | | | Ö - |

Drilling Method: Dual Rotary



PITEAU ASSOCIATES

GEOTECHNICAL AND HYDROGEOLOGICAL CONSULTANTS

Test Well: TW12-1 Project Number: 3131

Page 2 of 2

Location: Stave Lk Rd, Miracle Valley BC

| Depth Below Ground Surface | Elevation Lithologic Description | | Depth (mbg) | Lithology | Remarks | Constructed Well |
|---|-------------------------------------|--|----------------------|-----------|---|--------------------------|
| 132 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - | | SILTY SAND Brown silty fine to medium sand, with subrounded gravel SAND AND GRAVEL Brown-grey coarse sand and angular to rounded gravel, trace silt Since and angular to rounded gravel Sector and angular to rounded gravel GRAVELLY SAND Gravely coarse sand | 41.1 64.6 68.6 | | 203mm casing drilled to 76.8m and pulled back to 72.3m to expose screen Telescopic Stainless Steel Well Screen Assembly: | |
| 232 – 237 – - 73 242 – 247 – | | Colour change in water from brown to grey at 68m | 75.6 | | Exposed from 71.8 to 75.7m K-Packer and Solid Riser: 71.8 to 72.6m Screen Interval: 72.6 to 75.7m Slot Size: 2.03 mm | Riser K-packer Screen |
| 252 <u>-</u> 257 - 78 | | Grey sand with gravel, fining down to end of hole, trace silt End of Hole | 76.8 | | | Ative ground |

TW 12-1 Bbs well



н 1915 1917 1917

Report 1 - Detailed Well Record

| From 130 to 175 Ft. Sand and gravel | |
|--|--|
| From 0 to 20 Ft. Clay From 120 to 139 Ft. Till and clay | |
| LITHOLOGY INFORMATION: From 0 to 20 Ft. Clay | |
| | |
| GENERAL REMARKS: | |
| Casing from to feet | Diameter Material Drive Shoe |
| Screen from to feet | Type Slot Size |
| | Details of Closure: |
| Other Info Details: | Closure Backfill Material: |
| Other Info Flag: | Closure Sealant Material: |
| Site Info Details: | Method of Closure: |
| | Reason For Closure: |
| Screen Info Flag: | WELL CLOSURE INFORMATION: |
| Sieve Info Flag: | |
| File Info Flag: | Thickness (in): |
| Lithology Info Flag: | Depth (ft): |
| Bedrock Depth: feet | Method: |
| Well Cap Type: | Material: |
| Elevation: 0 feet (ASL) Final Casing Stick Up: inches | Flag: |
| Well Depth: 175 feet | SURFACE SEAL: |
| Casing drive shoe: | Water Supply System Well Name: |
| Diameter: 6.0 inches | Water Supply System Name: |
| Construction Method: Unknown Constru | Water Utility: |
| Observation Well Status: | |
| Observation Well Number: | Site Info (SEAM): |
| Well Use: Unknown Well Use | Field Chemistry Info Flag: |
| Status of Well: New | Water Chemistry Info Flag: Y |
| Orientation of Well: | EMS ID: |
| Subclass of Well: | Well Disinfected: N |
| Class of Well: | Odour: |
| | Colour: |
| BCGS Number (NAD 27): 092G029421 Well: 11 | Character: |
| Island: | WATER QUALITY: |
| Quarter: | |
| Indian Reserve: Meridian: Block: | Static Level: 116 feet |
| Township: 18 Section: 36 Range: | Artesian Pressure (ft): |
| District Lot: Plan: Lot: A | Artesian Flow: |
| NEW WESTMINSTER Land District | Pump Test Info Flag: |
| WELL LOCATION: | Development Method: |
| Area: | PRODUCTION DATA AT TIME OF DRILLING: Well Yield: 60 (Driller's Estimate) Gallons per Minute (U.S./Imperial) |
| Address: MIRACLE VALLEY | |
| | Where Plate Attached: |
| Owner: MIRACLE VALLEY CENTR | Driller: A. & H. Construction Well Identification Plate Number: |
| Well Tag Number: 33904 | |
| | Construction Date: 1975-12-01 00:00:00.0 |
| | |

Return to Main

- Return to Search Options
- Return to Search Criteria

Information Disclaimer

The Province disclaims all responsibility for the accuracy of information provided. Information provided should not be used as a basis for making financial or any other commitments.



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BC EWS Mouriening Well

Report 1 - Detailed Well Record

| | | gaaabaa baa baba 2002 | |
|--|---------|--------------------------|-------------------|
| Well Tag Number: 1190 | | Construction Date: 1901 | -01-01 00:00:00.0 |
| neii iug number, iiso | | Driller: Unknown | |
| Owner: RON ROSS | | Well Identification Pla | ate Number: |
| | | Plate Attached By: | |
| Address: 14100 STAVE LAKE RD. | | Where Plate Attached: | |
| | | | |
| Area: MISSION | | PRODUCTION DATA AT TIME | OF DRILLING: |
| | | Well Yield: 0 (Dril | ller's Estimate) |
| WELL LOCATION: | | Development Method: | |
| NEW WESTMINSTER Land District | | Pump Test Info Flag: N | |
| District Lot: Plan: 8404 Lot: | | Artesian Flow: | |
| Township: 18 Section: 36 Range: | | Artesian Pressure (ft): | : |
| Indian Reserve: Meridian: Block: | | Static Level: 92 feet | |
| Quarter: | | | |
| Island: | | WATER QUALITY: | |
| BCGS Number (NAD 27): 092G029421 We | ell: 14 | Character: | |
| | | Colour: | |
| Class of Well: Water supply | | Odour: | |
| Subclass of Well: Domestic | | Well Disinfected: N | |
| Orientation of Well: | | EMS ID: E217927 | |
| Status of Well: New | | Water Chemistry Info Fl | |
| Well Use: Water Supply System | | Field Chemistry Info Fl | .ag: |
| Observation Well Number: | | Site Info (SEAM): Y | |
| Observation Well Status: | | | |
| Construction Method: Drilled | | Water Utility: | |
| Diameter: 6.0 inches | | Water Supply System Nam | |
| Casing drive shoe: | | Water Supply System Wel | .1 Name: |
| Well Depth: 143.5 feet | | | |
| Elevation: 0 feet (ASL) | | SURFACE SEAL: | |
| Final Casing Stick Up: inches | | Flag: N | |
| Well Cap Type: | | Material: | |
| Bedrock Depth: feet | | Method: | |
| Lithology Info Flag: N | | Depth (ft): | |
| File Info Flag: N | | Thickness (in): | |
| Sieve Info Flag: N | | WELL CLOSURE INFORMATIC | NKT . |
| Screen Info Flag: N | | Reason For Closure: | /IN ; |
| Site Info Details: | | Method of Closure: | |
| Other Info Flag: | | Closure Sealant Materia | |
| Other Info Details: | | Closure Backfill Materia | |
| other into becarth. | | Details of Closure: | .ut i |
| Screen from to feet Ty | уре | Slot Size | |
| | iameter | Material | Drive Shoe |
| GENERAL REMARKS: | | | |
| GENERAL REMARKS: MIRACLE VALLEY REHABILITATION CENT | RE | | |
| | 4.11 | | |
| LITHOLOGY INFORMATION: | | | |

LITHOLOGY INFORMATION:

• Return to Main

<u>Return to Search Options</u>

APPENDIX C

DATA SUMMARY FOR AQUIFER TESTING WITH TW11-1

TABLE C-1 SUMMARY OF MANUAL DATA DURING VARIABLE-RATE PUMPING TEST WITH TW11-1

| | Clock | Elapsed | | Water | Levels | | | | | |
|-----------|---------|---------|-------|--------|--------|-------|-----------|----------------------|-------|---------------------------------|
| Date 2011 | Time | Time, t | Depth | bTOC | Draw | down | Ρι | umping Ra | te | Comments |
| | (h:m:s) | (min) | (ft) | (m) | (ft) | (m) | (USgpm) | (USgpm) (Igpm) (L/s) | | |
| 14-Dec | | | 82.89 | 25.265 | (14) | (11) | (oogpiii) | (igpiii) | (1/3) | STEP #1 |
| | 14:00 | 0 | 82.89 | | | | 1 | | | OBS WELL - 97.55 FT |
| | | 0.5 | 86.71 | 26.429 | 3.820 | 1.164 | 160 | 133 | 10.1 | |
| | | 1 | 87.15 | 26.563 | 4.260 | 1.298 | 1 | | | |
| | | 1.5 | 87.52 | 26.676 | 4.630 | 1.411 | 160 | 133 | 10.1 | |
| | | 2 | 87.52 | 26.676 | 4.630 | 1.411 | | | | WATER CLEAR |
| | | 2.5 | 87.52 | 26.676 | 4.630 | 1.411 | 160 | 133 | 10.1 | |
| | | 3 | 87.53 | 26.679 | 4.640 | 1.414 | | | | 22.5 IN AT ORIFICE |
| | | 3.5 | 87.53 | 26.679 | 4.640 | 1.414 | 160 | 133 | 10.1 | |
| | | 4 | 87.53 | 26.679 | 4.640 | 1.414 | | | | WATER CLEAR |
| | | 4.5 | 87.54 | 26.682 | 4.650 | 1.417 | 160 | 133 | 10.1 | |
| | 14:05 | 5 | 87.54 | 26.682 | 4.650 | 1.417 | | | | 22.5 IN AT ORIFICE |
| | | 6 | 87.54 | 26.682 | 4.650 | 1.417 | 160 | 133 | 10.1 | |
| | | 7 | 87.54 | 26.682 | 4.650 | 1.417 | | | | WATER CLEAR |
| | | 8 | 87.54 | 26.682 | 4.650 | 1.417 | 160 | 133 | 10.1 | 34.6 USGPF |
| | | 9 | 87.54 | 26.682 | 4.650 | 1.417 | | | | |
| | 14:10 | 10 | 87.54 | 26.682 | 4.650 | 1.417 | 160 | 133 | 10.1 | |
| | | 12 | 87.55 | 26.685 | 4.660 | 1.420 | | | | 22.5 IN AT ORIFICE |
| | | 14 | 87.55 | 26.685 | 4.660 | 1.420 | 160 | 133 | 10.1 | WATER CLEAR |
| | | 16 | 87.55 | 26.685 | 4.660 | 1.420 | | | | |
| | | 18 | 87.55 | | | | 160 | 133 | 10.1 | 22.5 IN AT ORIFICE |
| | 14:20 | 20 | 87.55 | 26.685 | 4.660 | 1.420 | | | | OBS WELL 97.62 FT |
| | | 25 | 87.61 | 26.704 | 4.720 | 1.439 | 160 | 133 | 10.1 | |
| | 14:30 | 30 | 87.61 | 26.704 | 4.720 | 1.439 | | | | CHANGED PLATE TO 5 IN |
| | | | | | | | | | | SHUT DOWN DUE TO GENERATOR |
| | | | | | | | | | | PROBLEMS STEP CONTINUE NEXT DAY |
| | | | | | | | | | | STEP #2 - DEC 15, 2011 |
| 15-Dec | 8:50 | 0 | 83.00 | 25.298 | 0.110 | 0.034 | | | | OBS WELL - 97.69 FT |
| 10-000 | 0.00 | 0.5 | 87.85 | 26.777 | 4.960 | 1.512 | 176 | 147 | 11.1 | 000 WEEL - 37.0311 |
| | | 0.0 | 07.00 | 20.111 | 4.000 | 1.012 | | 1-17 | | VALVING |
| | | 1.5 | 92.66 | 28.243 | 9.770 | 2.978 | 338 | 282 | 21.3 | WILLING . |
| | | 2 | 92.75 | 28.270 | 9.860 | 3.005 | 000 | 202 | 2110 | WATER CLEAR |
| | | 2.5 | 92.79 | 28.282 | 9.900 | 3.018 | 338 | 282 | 21.3 | |
| | | 3 | 92.80 | 28.285 | 9.910 | 3.021 | | | | 6 IN AT ORIFICE |
| | | 3.5 | 92.82 | 28.292 | 9.930 | 3.027 | 338 | 282 | 21.3 | |
| | | 4 | 92.82 | 28.292 | 9.930 | 3.027 | | | - | |
| | | 4.5 | 92.84 | 28.298 | 9.950 | 3.033 | 338 | 282 | 21.3 | WATER CLEAR |
| | 8:55 | 5 | 92.86 | 28.304 | 9.970 | 3.039 | | | | |
| | | 6 | 92.87 | 28.307 | 9.980 | 3.042 | 338 | 282 | 21.3 | |
| | | 7 | 92.89 | 28.313 | 10.000 | 3.048 | | | | |
| | | 8 | 92.89 | 28.313 | 10.000 | 3.048 | 338 | 282 | 21.3 | |
| | | 9 | 92.89 | 28.313 | 10.000 | 3.048 | | | | PH 7.07 - TEMP 11.0°C |
| | 9:00 | 10 | 92.90 | 28.316 | 10.010 | 3.051 | 338 | 282 | 21.3 | COND. 1010 |
| | | 12 | 92.90 | 28.316 | 10.010 | 3.051 | | | | TDS 520 |
| | | 14 | 92.90 | | | | 338 | 282 | 21.3 | |
| | | 16 | 92.91 | | | | | | | WATER CLEAR |
| | | 18 | 92.92 | 28.322 | 10.030 | 3.057 | 338 | 282 | 21.3 | |
| | 9:10 | 20 | 92.93 | 28.325 | 10.040 | 3.060 | | | | OBS WELL 97.85 FT |
| | | 25 | 92.92 | 28.322 | 10.030 | 3.057 | 338 | 282 | 21.3 | |
| | 9:20 | 30 | 92.92 | 28.322 | 10.030 | 3.057 | | | | |

TABLE C-1 SUMMARY OF MANUAL DATA DURING VARIABLE-RATE PUMPING TEST WITH TW11-1

| | Clock | Elapsed | | Water | Levels | | | | | |
|-----------|---------|------------|------------------|------------------|------------------|----------------|---------|-----------|-------|------------------------|
| Date 2011 | Time | Time, t | Depth | bTOC | Draw | down | Pu | umping Ra | te | Comments |
| | (h:m:s) | (min) | (ft) | (m) | (ft) | (m) | (USgpm) | (Igpm) | (L/s) | 1 |
| | | | | | | | | | | STEP #3 |
| | | 30.5 | 07.40 | | | | 170 | 000 | 00.7 | VALVING |
| | | 31 31.5 | 97.46 97.49 | 29.715 | 14.600 | 4.450 | 472 | 393 | 29.7 | |
| | | 31.5 | 97.52 | 29.724 | 14.630 | 4.459 | 470 | 392 | 29.6 | |
| | | 32.5 | 97.51 | 29.721 | 14.620 | 4.456 | | | | |
| | | 33 | 97.52 | 29.724 | 14.630 | 4.459 | 470 | 392 | 29.6 | 13 IN AT ORIFICE |
| | | 33.5 | 97.53 | 29.727 | 14.640 | 4.462 | | | | |
| | | 34 | 97.56 | 29.736 | 14.670 | 4.471 | 470 | 392 | 29.6 | WATER CLEAR |
| | 9:25 | 34.5 35 | 97.55 | | | | 470 | 392 | 29.6 | |
| | 9.20 | 36 | 97.56 | 29.736 | 14.670 | 4.471 | 470 | 332 | 23.0 | |
| | | 37 | 97.57 | 29.739 | 14.680 | 4.474 | 470 | 392 | 29.6 | |
| | | 38 | 97.58 | 29.742 | 14.690 | 4.478 | | | | WATER CLEAR |
| | | 39 | 97.58 | 29.742 | 14.690 | 4.478 | 470 | 392 | 29.6 | 13 IN AT ORIFICE |
| | 9:30 | 40 | 97.59 | 29.745 | 14.700 | 4.481 | | 000 | 00.0 | |
| | | 42 44 | 97.60 97.62 | 29.748 29.755 | 14.710 14.730 | 4.484 4.490 | 470 | 392 | 29.6 | 13 IN AT ORIFICE |
| | | 44 | 97.62 | 29.755 | 14.730 | 4.490 | 470 | 392 | 29.6 | OBS WELL 97.90 FT |
| | | 40 | 97.62 | 20.100 | 1.1.30 | | 410 | 0.92 | 23.0 | PH 8.22 |
| | 9:40 | 50 | 97.65 | 29.764 | 14.760 | 4.499 | 470 | 392 | 29.6 | COND 1033 |
| | | 55 | 97.67 | | | | | | | TDS 520 |
| | 9:50 | 60 | 97.68 | 29.773 | 14.790 | 4.508 | 470 | 392 | 29.6 | TEMP 8.1°C |
| | | | | | | | | | | STEP #4 |
| | | 60.5 61 | 100.42 | 30.608 | 17.530 | 5.343 | 548 | 457 | 34.5 | VALVING |
| | | 61.5 | 100.42 | 30.000 | 17.550 | 5.343 | 540 | 437 | 34.5 | WATER CLEAR |
| | | 62 | 100.42 | 30.617 | 17.560 | 5.352 | 550 | 458 | 34.7 | 17.5 IN AT ORIFICE |
| | | 62.5 | 100.45 | 30.617 | 17.560 | 5.352 | | | • | |
| | | 63 | 100.45 | 30.617 | 17.560 | 5.352 | 550 | 458 | 34.7 | |
| | | 63.5 | 100.46 | 30.620 | 17.570 | 5.355 | | | | 17.5 IN AT ORIFICE |
| | | 64 | 100.47 | 30.623 | 17.580 | 5.358 | 550 | 458 | 34.7 | |
| | 9:55 | 64.5 65 | 100.48 100.49 | 30.626 30.629 | 17.590 17.600 | 5.361 5.364 | 550 | 458 | 34.7 | WATER CLEAR |
| | 9.00 | 66 | 100.43 | 30.638 | 17.630 | 5.374 | 550 | 400 | 54.7 | |
| | | 67 | 100.55 | 30.648 | 17.660 | 5.383 | 550 | 458 | 34.7 | 17.5 IN AT ORIFICE |
| | | 68 | 100.55 | 30.648 | 17.660 | 5.383 | | | | |
| | | 69 | 100.56 | 30.651 | 17.670 | 5.386 | 550 | 458 | 34.7 | WATER CLEAR |
| | 10:00 | 70 | 100.56 | 30.651 | 17.670 | 5.386 | 550 | 450 | 017 | |
| | | 72 74 | 100.57 100.57 | 30.654 30.654 | 17.680 17.680 | 5.389 5.389 | 550 | 458 | 34.7 | PH - 7.96 |
| | | 74 | 100.57 | 30.654 | 17.670 | 5.386 | 550 | 458 | 34.7 | COND - 899, TEMP 7.4°C |
| | | 78 | 100.58 | 30.657 | 17.690 | 5.392 | 000 | | 01.7 | TDS - 440 |
| | 10:10 | 80 | 100.57 | 30.654 | 17.680 | 5.389 | 550 | 458 | 34.7 | OBS WELL 97.98 FT |
| | | 85 | 100.64 | 30.675 | 17.750 | 5.410 | | | | |
| | 10:20 | 90 | 100.65 | 30.678 | 17.760 | 5.413 | 550 | 458 | 34.7 | |
| | | 90.5 | 83.40 | 25.420 | 0.510 | 0.155 | | | | RECOVERY |
| | | 90.5 | 83.30 | 25.390 | 0.510 | 0.155 | | | | |
| | | 91.5 | 83.25 | 25.375 | 0.360 | 0.120 | | | | |
| | | 92 | 83.23 | | 0.340 | 0.104 | | | | |
| | | 92.5 | 83.20 | 25.359 | 0.310 | 0.094 | | | | |
| | | 93 | 83.16 | 25.347 | 0.270 | 0.082 | | | | |
| | | 93.5 94 | 83.15 83.15 | 25.344 25.344 | 0.260 | 0.079 | | | | |
| | | 94 | 83.15 | 25.344 | 0.260 | 0.079 | | | | |
| | 10:25 | 95 | 83.14 | 25.341 | 0.250 | 0.076 | | | | |
| | | 96 | 83.13 | 25.338 | 0.240 | 0.073 | | | | |
| | 10:27 | 97 | 83.13 | 25.338 | 0.240 | 0.073 | | | | |
| | 10:32 | 102 | 83.11 | 25.332 | 0.220 | 0.067 | | | | |
| | 10:44 | 114 | 83.01 | 25.301 | 0.120 | 0.037 | | | | |
| | 11:20 | 150 | 83.03 | 25.308 | 0.140 | 0.043 | | | | END RECOVERY |

TABLE C-2 SUMMARY OF MANUAL DATA DURING CONSTANT-RATE PUMPING TEST WITH TW11-1

| | Clock | Elapsed | | Water | Levels | | | | | |
|-----------|----------------|----------------|------------------|------------------|------------------|----------------|----------------|------------|--------------|---|
| Date 2011 | Time | Time, t | Depth | bTOC | Draw | down | P | umping Ra | te | Comments |
| | (h:m:s) | (min) | (ft) | (m) | (ft) | (m) | (USgpm) | (Igpm) | (L/s) | - |
| 15-Dec | 11:30 | 0 | 83.04 | 25.311 | 0.000 | 0.000 | (y / | | | STATIC |
| | | 0.5 | | | | | | | | OBS WELL 97.76 FT |
| | | 1 | 93.75 | 28.575 | 10.710 | 3.264 | | | | VALVING |
| | | 1.5 2 | 95.32 97.71 | 29.054 29.782 | 12.280 14.670 | 3.743 4.471 | 527 | 439 | 33.2 | |
| | | 2.5 | 100.00 | 30.480 | 16.960 | 5.169 | 521 | 400 | 55.Z | WATER BROWNISH COLOUR |
| | | 3 | 100.15 | 30.526 | 17.110 | 5.215 | 550 | 458 | 34.7 | |
| | | 3.5 | 100.17 | 30.532 | 17.130 | 5.221 | | | | WATER CLEAR |
| | | 4 | 100.23 | 30.550 | 17.190 | 5.240 | 549 | 457 | 34.6 | |
| | 44.05 | 4.5 | 100.25 | 30.556 | 17.210 | 5.246 | 5.40 | | | 17.5 IN AT ORIFICE |
| | 11:35 | 5 | 100.26 100.30 | 30.559 30.571 | 17.220 17.260 | 5.249 5.261 | 549 | 457 | 34.6 | |
| | | 7 | 100.30 | 30.590 | 17.320 | 5.279 | | | | |
| | | 8 | 100.35 | 30.587 | 17.310 | 5.276 | | | | |
| | | 9 | 100.36 | 30.590 | 17.320 | 5.279 | 549 | 457 | 34.6 | |
| | 11:40 | 10 | 100.37 | 30.593 | 17.330 | 5.282 | | | | |
| | | 12 | 100.41 | 30.605 | 17.370 | 5.294 | 5.40 | | | |
| | | 14 | 100.44 | 30.614 | 17.400 | 5.304 | 549 | 457 | 34.6 | |
| | | 16 18 | 100.45 100.46 | 30.617 30.620 | 17.410 17.420 | 5.307 5.310 | | | | 17.5 IN AT ORIFICE |
| | 11:50 | 20 | 100.40 | 00.020 | 11.720 | 0.010 | 1 | | | WATER CLEAR |
| | | 25 | 100.48 | 30.626 | 17.440 | 5.316 | 549 | 457 | 34.6 | |
| | 12:00 | 30 | 100.52 | 30.638 | 17.480 | 5.328 | | | | |
| | | 35 | 100.52 | 30.638 | 17.480 | 5.328 | 549 | 457 | 34.6 | |
| | 12:10 | 40 | 100.55 | 30.648 | 17.510 | 5.337 | | 40- | 04 - | |
| | 12:20 | 50 | 100.59 | 30.660 | 17.550 | 5.349 | 548 | 457 | 34.5 | |
| | 12:30 12:40 | 60 70 | 100.61 100.61 | 30.666 30.666 | 17.570 17.570 | 5.355 5.355 | | | | OBS WELL 98.01 FT |
| | 12:40 | 80 | 100.60 | 30.663 | 17.560 | 5.355 | 549 | 457 | 34.6 | |
| | 13:00 | 90 | 100.61 | 30.666 | 17.570 | 5.355 | 0.0 | | | |
| | 13:10 | 100 | 100.66 | 30.681 | 17.620 | 5.371 | 548 | 457 | 34.5 | |
| | 13:30 | 120 | 100.69 | 30.690 | 17.650 | 5.380 | | | | PH - 7.9, COND - 345, TDS-480 |
| | 13:50 | 140 | 100.68 | 30.687 | 17.640 | 5.377 | 549 | 457 | 34.6 | TEMP - 6.8°C |
| | 14:10 | 160 | 100.72 | 30.699 | 17.680 | 5.389 | | | | OBS WELL 98.05 FT |
| | 14:30 14:50 | 180 200 | 100.75 100.75 | 30.709 30.709 | 17.710 17.710 | 5.398 5.398 | 548 | 457 | 34.5 | PH - 5.88, COND - 314, TDS-680 TEMP - 6.7°C |
| | 15:40 | 250 | 100.74 | 30.706 | 17.700 | 5.395 | 548 | 457 | 34.5 | OBS WELL 98.08 FT |
| | 16:30 | 300 | 100.77 | 30.715 | 17.730 | 5.404 | | | | PH-5.8, TDS-243, COND124, TEMP 6.9°C |
| | 17:20 | 350 | 100.86 | 30.742 | 17.820 | 5.432 | 550 | 458 | 34.7 | 17.5 IN AT ORIFICE, TWEAKED VFD |
| | 18:10 | 400 | 100.89 | 30.751 | 17.850 | 5.441 | | | | OBS WELL 98.06 FT |
| | 19:00 | 450 | 100.92 | 30.760 | 17.880 | 5.450 | | 450 | 047 | PH-5.57, COND- 570, TDS-270, TEMP 6.0°C |
| | 19:50 20:40 | 500 550 | 100.92 100.92 | 30.760 30.760 | 17.880 17.880 | 5.450 5.450 | 550 | 458 | 34.7 | |
| | 20.40 | 600 | 100.92 | 30.760 | 17.890 | 5.450 | | | | TEMP 6.0°C, PH-5.26, COND-1080, TDS-542 |
| | 22:20 | 650 | 100.94 | 30.767 | 17.900 | 5.456 | 549 | 457 | 34.6 | OBS WELL - 97.17 FT |
| | 23:10 | 700 | 100.94 | | | | | | | 17.5 IN AT ORIFICE |
| 16-Dec | 0:00 | 750 | 100.93 | | | | | | | DEC 16, 11 - WATER CLEAR |
| | 0:50 | 800 | 100.92 | 30.760 | 17.880 | 5.450 | | | | TEMP 6.0°C, PH-5.46 TDS-582 |
| | 1:40 | 850 | 100.91 | 30.757 | 17.870 | 5.447 | | | | 17.5 IN AT ORIFICE |
| | 2:30 3:20 | 900 950 | 100.92 100.94 | 30.760 30.767 | 17.880 17.900 | 5.450 5.456 | | | | TEMP 6.0°C, PH-5.29, COND-531, TDS-468 OBS WELL - 98.19 FT |
| | 4:10 | 1000 | 100.94 | 30.767 | 17.900 | 5.456 | 1 | | | |
| | 5:00 | 1050 | 100.94 | 30.767 | 17.900 | 5.456 | 1 | | 1 | PH-7.05, COND-592, TDS-341, TEMP 6.3°C |
| | 5:50 | 1100 | 100.92 | | | | 550 | 458 | 34.7 | 17.5 IN AT ORIFICE |
| | 6:40 | 1150 | 100.92 | 30.760 | 17.880 | 5.450 | 551 | 459 | 34.7 | OBS WELL 98.2 FT |
| | 7:30 | 1200 | 100.91 | 30.757 | 17.870 | 5.447 | | 450 | 047 | PH-7.05, COND-216, TDS-111, TEMP 6.9°C |
| | 8:20 9:10 | 1250 1300 | 100.90 100.91 | 30.754 30.757 | 17.860 17.870 | 5.444 5.447 | 550 550 | 458 458 | 34.7 34.7 | 17.5 IN AT ORIFICE OBS WELL 98.25 FT |
| | 10:00 | 1300 | 100.91 | 30.757 | 17.870 | 5.447 | 550 | 400 | 34.7 | OBS WELL 98.25 FT PH-7.21, COND-172, TDS-159, TEMP 9.9°C |
| | 10:50 | 1400 | 100.92 | 30.760 | 17.880 | 5.450 | 1 | | | 17.5 IN AT ORIFICE |
| | 11:30 | 1440 | | | | | | | | OBS WELL 98.23 FT |
| | | | | | | | | | | RECOVERY |
| | | 1440.5 | 83.40 | 25.420 | 0.360 | 0.110 | | | | |
| | | 1441 | 83.52 | 25.457 | 0.480 | 0.146 | | | | |
| | | 1441.5 1442 | 83.48 83.43 | 25.445 25.429 | 0.440 0.390 | 0.134 0.119 | | | | |
| | | 1442.5 | 83.43 | 25.429 | 0.390 | 0.119 | | (| | |
| | | 1443 | 83.38 | 25.414 | 0.340 | 0.104 | 1 | | | |
| | | 1443.5 | 83.40 | | 0.360 | 0.110 | 1 | | | |
| | | 1444 | 83.38 | 25.414 | 0.340 | 0.104 | | | | |
| | | 1444.5 | | | | | | | | |
| | 11:35 | 1445 | 83.37 | 25.411 | 0.330 | 0.101 | | | | |
| | | 1446 1447 | 83.41 83.42 | 25.426 | 0.380 | 0.116 | | | | |
| | | 1447 1448 | 83.42 | 25.426 | 0.380 | 0.116 0.110 | 1 | | | |
| | | 1448 | 83.40 | | 0.360 | 0.110 | 1 | | | |
| | 11:40 | 1450 | 83.39 | | 0.350 | 0.107 | 1 | | | |
| | 11:51 | 1461 | 83.35 | | | | 1 | | | |
| | 12:02 | 1472 | 83.30 | | 0.260 | 0.079 | | | | |
| | 12:15 | 1485 | 83.30 | 25.390 | 0.260 | 0.079 | | | | OBS WELL 98.31 FT AT 12:45 |
| | 13:30 | 1500 | 83.27 | 25.381 | 0.230 | 0.070 | | | | END RECOVERY |

APPENDIX D

DATA SUMMARY FOR AQUIFER TESTING WITH TW12-1

TABLE D-1 SUMMARY OF MANUAL DATA DURING VARIABLE-RATE PUMPING TEST WITH TW12-1

| | Clock | Elapsed | | Water | Levels | | _ | | 4- | |
|-----------|---------|------------|------------------|------------------|----------------|----------------|------------|------------|--------------|-----------------------------------|
| Date 2012 | Time | Time, t | Depth | bTOC | Draw | down | _ Pi | umping Ra | te | Comments |
| | (h:m:s) | (min) | (ft) | (m) | (ft) | (m) | (USgpm) | (Igpm) | (L/s) | |
| 17-Jan | 10:30 | 0 | 116.61 | 35.543 | 0.000 | 0.000 | | | | STEP #1 |
| | | 0.5 | 117.45 | | | | | | | 6 IN AT ORIFICE = 161 USGPM |
| | | 1 | 107.20 | 38.801 | 10 600 | 3.258 | 116 | 97 | 7.2 | |
| | | 1.5 2 | 127.30 | 38.801 | 10.690 | 3.258 | 110 | 97 | 7.3 | WATER AT ORIFICE VALVE BACK |
| | | 2.5 | 118.10 | 35.997 | 1.490 | 0.454 | 143 | 119 | 9.0 | VFD UP |
| | | 3 | 118.00 | 35.966 | 1.390 | 0.424 | 144 | 120 | 9.1 | VALVE BACK |
| | | 3.5 | | | | | | | | |
| | | 4 | 118.10 | 35.997 | 1.490 | 0.454 | 163 | 136 | 10.3 | |
| | 40.25 | 4.5 | 110.10 | 25.007 | 1 400 | 0 454 | | | | |
| | 10:35 | 5 6 | 118.10 118.10 | 35.997 35.997 | 1.490 1.490 | 0.454 | | | | WATER VERY BROWN |
| | | 7 | 118.10 | 35.997 | 1.490 | 0.454 | | | | |
| | | 8 | 118.10 | 35.997 | 1.490 | 0.454 | | | | |
| | | 9 | 118.10 | 35.997 | 1.490 | 0.454 | | | | |
| | 10:40 | 10 | 118.10 | 35.997 | 1.490 | 0.454 | | | | |
| | | 12 | 118.10 | 35.997 | 1.490 | 0.454 | 400 | 400 | 40 F | |
| | | 14 16 | 118.13 118.15 | 36.006 36.012 | 1.520 1.540 | 0.463 | 166 167 | 139 139 | 10.5 10.5 | 6.5 IN AT ORIFICE |
| | | 16 | 118.15 | 36.012 | 1.540 | 0.469 | 167 | 139 | 10.5 | |
| | 10:50 | 20 | 118.15 | 36.012 | 1.540 | 0.469 | 167 | 139 | 10.5 | |
| | | 25 | 118.15 | 36.012 | 1.540 | 0.469 | 169 | 140 | 10.6 | VALVED BACK TO MAINTAIN FLOW |
| | 11:00 | 30 | 118.15 | 36.012 | 1.540 | 0.469 | 167 | 139 | 10.5 | 105.84 USGPF |
| | | | 440.00 | 00 150 | 0.000 | 0.010 | | | | STEP #2 - VALVE UP |
| | 1 | 30.5 31 | 118.63 119.05 | 36.158 36.286 | 2.020 2.440 | 0.616 | 234 | 195 | 14.7 | 22 IN AT ORIFICE |
| | | 31 | 119.05 | 36.286 | 2.440 | 0.744 | 234 275 | 229 | 14.7 | VALVE UP |
| | | 32 | 119.35 | 36.378 | 2.740 | 0.835 | 288 | 240 | 18.1 | VFD UP |
| | | 32.5 | 119.51 | 36.427 | 2.900 | 0.884 | | | | |
| | | 33 | | | | | 325 | 271 | 20.5 | |
| | | 33.5 | 119.95 | 36.561 | 3.340 | 1.018 | 305 | 254 | 19.2 | VALVE BACK |
| | | 34 | 119.75 | 36.500 36.500 | 3.140 | 0.957 | 205 | 254 | 10.0 | |
| | 11:05 | 34.5 35 | 119.75 119.76 | 36.500 | 3.140 3.150 | 0.957 | 305 312 | 254 | 19.2 19.7 | 20.5 IN AT ORIFICE VALVE UP |
| | 11.00 | 36 | 119.70 | 36.485 | 3.090 | 0.942 | 315 | 262 | 19.8 | |
| | | 37 | 119.87 | 36.536 | 3.260 | 0.994 | | | | 22 IN AT ORIFICE |
| | | 38 | 119.87 | 36.536 | 3.260 | 0.994 | 315 | 262 | 19.8 | WATER CLEAR |
| | | 39 | 119.97 | 36.567 | 3.360 | 1.024 | 315 | 262 | 19.8 | 22 IN AT ORIFICE |
| | 11:10 | 40 | 119.87 | 36.536 | 3.260 | 0.994 | 316 | 263 | 19.9 | COLD OUT SIDE |
| | | 42 44 | 119.87 119.88 | 36.536 36.539 | 3.260 3.270 | 0.994 | 315 | 263 | 19.9 | |
| | | 44 | 119.88 | 36.539 | 3.270 | 0.997 | | | | |
| | | 48 | 119.88 | 36.539 | 3.270 | 0.997 | | | | 22 IN AT ORIFICE |
| | 11:20 | 50 | 119.88 | 36.539 | 3.270 | 0.997 | | | | |
| | | 55 | 119.88 | 36.539 | 3.270 | 0.997 | 315 | 263 | 19.8 | |
| | 11:30 | 60 | 119.95 | 36.561 | 3.340 | 1.018 | 316 | 263 | 19.9 | CHANGE TO 5 IN PLATE |
| | | 60.5 | 120.35 | 36.683 | 3.740 | 1.140 | 340 | 283 | 21.4 | STEP #3 - VALVE UP 449 USGPM |
| | | 61 | 120.00 | 00.000 | 0.740 | 1.140 | 0-10 | 200 | 21.4 | 13 IN AT ORIFICE |
| | | 61.5 | 121.45 | 37.018 | 4.840 | 1.475 | 443 | 369 | 27.9 | VFD UP |
| | | 62 | 121.49 | 37.030 | 4.880 | 1.487 | 443 | 369 | 27.9 | |
| | | 62.5 | 121.56 | | 4.950 | 1.509 | 443 | 369 | 27.9 | VALVE UP |
| | | 63 | 121.60 | 37.064 | 4.990 | 1.521 | 445 | 371 | 28.0 | |
| | | 63.5 64 | 121.70 | 37.094 | 5.090 | 1.551 | - | | | ADJUSTING VALVE |
| | | 64.5 | | | | | | | | |
| | 11:35 | 65 | 121.70 | 37.094 | 5.090 | 1.551 | 451 | 376 | 28.4 | 11 IN AT ORIFICE |
| | | 66 | 121.70 | 37.094 | 5.090 | 1.551 | | | | VALVE UP |
| | | 67 | 121.79 | 37.122 | 5.180 | 1.579 | 461 | 384 | 29.0 | |
| | | 68 | 400.00 | 07.050 | F 040 | 4 740 | 400 | 400 | 00.0 | VALVE UP 13 IN AT ORIFICE |
| | 11:40 | 69 70 | 122.22 122.25 | 37.253 37.262 | 5.610 5.640 | 1.710 1.719 | 490 490 | 409 408 | 30.9 | |
| | 11:40 | 70 | 122.25 | 37.262 | 5.640 | 1.719 | 490 | 408 | 30.9 30.9 | WATER CLEAR |
| | | 74 | 122.25 | 37.262 | 5.640 | 1.719 | 100 | 100 | 00.0 | 13 IN. AT ORIFICE |
| | | 76 | 122.28 | 37.271 | 5.670 | 1.728 | 490 | 408 | 30.9 | |
| | | 78 | 122.30 | 37.277 | 5.690 | 1.734 | | | | |
| | 11:50 | 80 | 122.30 | 37.277 | 5.690 | 1.734 | | | | |
| | 12:00 | 85 90 | 122.30 | 37.277 | 5.690 | 1.734 1.734 | 488 | 407 | 30.7 | |
| | 12:00 | 90 | 122.30 | 37.277 | 5.690 | 1.734 | | | | 80.32 USGPF STEP #4 - VALVE UP |
| | | 90.5 | | | | | | | | 16.5 IN AT ORIFICE |
| | | 91 | | | | | | | | |
| | | 91.5 | | | | | | | | |
| | | 92 | 123.10 | 37.521 | 6.490 | 1.978 | 549 | 457 | 34.6 | VALVE UP |
| | | 92.5 | 123.12 | 37.527 | 6.510 | 1.984 | | | | 16 IN AT ORIFICE |
| | | 93 93.5 | 123.15 | 37.536 | 6.540 | 1.993 | | | | VALVE UP |
| | | 93.5 | 123.15 | 37.536 | 6.540 | 1.993 | | | | |
| | | 94.5 | 123.13 | | 6.570 | 2.003 | | | | WATER CLEAR |

TABLE D-1 SUMMARY OF MANUAL DATA DURING VARIABLE-RATE PUMPING TEST WITH TW12-1

| | Clock | Elapsed | | Water | Levels | | | | 4.0 | |
|-----------|---------|---------|--------|--------|--------|-------|---------|--------------|-------|-------------------------------------|
| Date 2012 | Time | Time, t | Depth | bTOC | Draw | down | P | Pumping Rate | | Comments |
| | (h:m:s) | (min) | (ft) | (m) | (ft) | (m) | (USgpm) | (Igpm) | (L/s) | - |
| | 12:05 | 95 | 123.18 | 37.545 | 6.570 | 2.003 | 550 | 459 | 34.7 | |
| | | 96 | 123.18 | 37.545 | 6.570 | 2.003 | | | | |
| | | 97 | 123.18 | 37.545 | 6.570 | 2.003 | | | | VALVE UP |
| | | 98 | 123.25 | 37.567 | 6.640 | 2.024 | 557 | 464 | 35.1 | 17 IN MAXED ON PUMP |
| | | 99 | 123.33 | 37.591 | 6.720 | 2.048 | | | | |
| | 12:10 | 100 | 123.33 | 37.591 | 6.720 | 2.048 | | | | |
| | | 102 | 123.33 | 37.591 | 6.720 | 2.048 | | | | |
| | | 104 | 123.28 | 37.576 | 6.670 | 2.033 | | | | |
| | | 106 | 123.28 | 37.576 | 6.670 | 2.033 | | | | µs 120, PPM 60, PH 8.79, TEMP 6.6°C |
| | | 108 | 123.34 | 37.594 | 6.730 | 2.051 | 555 | 462 | 35.0 | |
| | 12:20 | 110 | 123.33 | 37.591 | 6.720 | 2.048 | | | | |
| | | 115 | 123.35 | 37.597 | 6.740 | 2.054 | 557 | 464 | 35.1 | |
| | 12:30 | 120 | 123.35 | 37.597 | 6.740 | 2.054 | 557 | 464 | 35.1 | 76.11 USGPF |
| | | | | | | | | | | RECOVERY |
| | | 120.5 | 117.36 | 35.771 | 0.750 | 0.229 | | | | |
| | | 121 | 116.99 | 35.659 | 0.380 | 0.116 | | | | |
| | | 121.5 | 116.99 | 35.659 | 0.380 | 0.116 | | | | |
| | | 122 | 116.97 | 35.652 | 0.360 | 0.110 | | | | |
| | | 122.5 | 116.95 | 35.646 | 0.340 | 0.104 | | | | |
| | | 123 | 116.93 | 35.640 | 0.320 | 0.098 | | | | |
| | | 123.5 | 116.91 | 35.634 | 0.300 | 0.091 | | | | |
| | | 124 | 116.90 | 35.631 | 0.290 | 0.088 | | | | |
| | | 124.5 | 116.89 | 35.628 | 0.280 | 0.085 | | | | |
| | 12:35 | 125 | 116.85 | 35.616 | 0.240 | 0.073 | | | | |
| | | 126 | 116.85 | 35.616 | 0.240 | 0.073 | | | | |
| | | 127 | 116.85 | 35.616 | 0.240 | 0.073 | | | | |
| | | 128 | 116.83 | 35.610 | 0.220 | 0.067 | | | | |
| | | 129 | 116.83 | 35.610 | 0.220 | 0.067 | | | | |
| | 12:40 | 130 | 116.82 | 35.607 | 0.210 | 0.064 | | | | |
| | | 132 | | | | | | | | |
| | | 134 | 116.78 | 35.595 | 0.170 | 0.052 | | | | |
| | | 136 | | | | | | | | |
| | | 138 | 116.77 | 35.591 | 0.160 | 0.049 | | | | |
| | 12:50 | 140 | | | | | | | | |
| | | 145 | | | | | | | | |
| | 13:00 | 150 | 116.75 | 35.585 | 0.140 | 0.043 | | | | END RECOVERY |

TABLE D-2 SUMMARY OF MANUAL DATA DURING CONSTANT-RATE PUMPING TEST WITH TW12-1

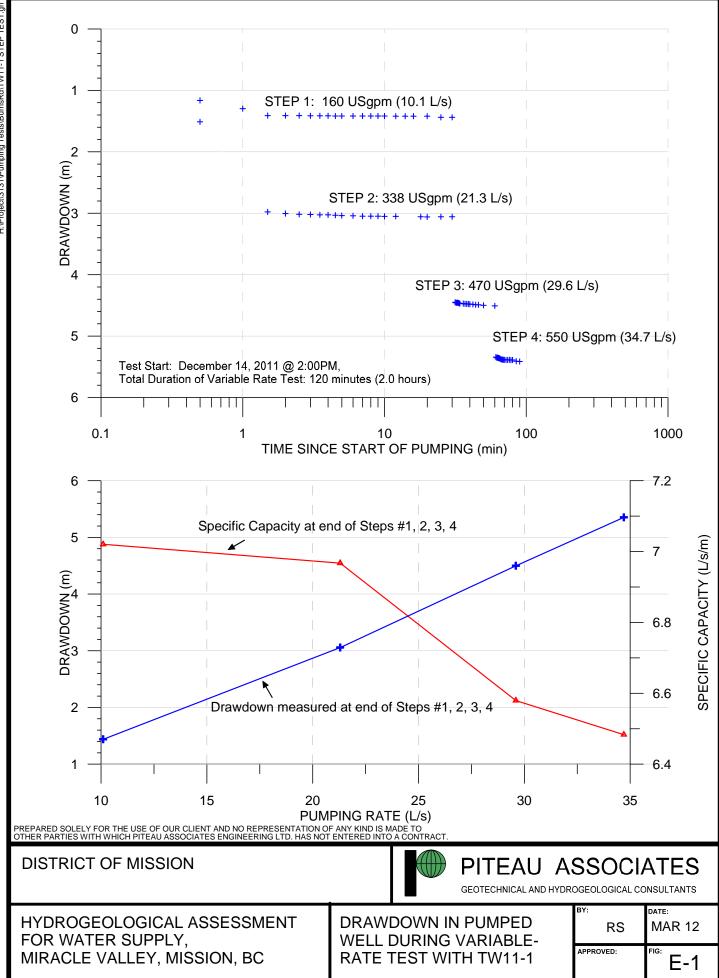
| | Clock | Elapsed | | Water | Levels | | | | | |
|-----------|----------------|------------|------------------|------------------|----------------|----------------|------------|------------|--------------|--|
| Date 2012 | Time | Time, t | Depth | | | down | Pu | umping Ra | te | Comments |
| | (h:m:s) | (min) | (ft) | (m) | (ft) | (m) | (USgpm) | (Igpm) | (L/s) | |
| 17-Jan | 14:30 | 0 | 116.72 | 35.576 | 0.000 | 0.000 | (oogpiii) | (igpiii) | (6/3) | STATIC |
| | | 0.5 | 123.00 | 37.490 | 6.280 | 1.914 | | | | WATER AT ORIFICE |
| | | 1 | 123.01 | 37.493 | 6.290 | 1.917 | | | | WATER BROWN |
| | | 1.5 | 123.08 | 37.515 | 6.360 | 1.939 | 565 | 471 | 35.6 | |
| | | 2 | 123.19 | 37.548 | 6.470 | 1.972 | | | | |
| | | 2.5 | 123.13 | 37.530 | 6.410 | 1.954 | 565 | 471 | 35.6 | |
| | | 3 | 123.19 | 37.548 | 6.470 | 1.972 | | | | |
| | | 3.5 | 123.21 | 37.554 | 6.490 | 1.978 | | | | |
| | | 4 | 123.25 | 37.567 | 6.530 | 1.990 | 565 | 471 | 35.6 | |
| | 44.05 | 4.5 | 123.28 | 37.576 | 6.560 | 1.999 | 550 | 450 | 047 | |
| | 14:35 | 5 | 123.23 123.23 | 37.561 37.561 | 6.510 | 1.984 1.984 | 550 | 458 | 34.7 | CHECK ORIFICE WATER FROZEN IN PIZO. |
| | | 6 7 | 123.23 | 37.501 | 6.510 | 1.964 | | | | WATER FROZEN IN PIZO. |
| | | 8 | 123.23 | 37.561 | 6.510 | 1.984 | | | | FIX PIZO. TUBE |
| | | 0 9 | 123.23 | 37.573 | 6.550 | 1.984 | | | | |
| | 14:40 | 10 | 123.21 | 37.554 | 6.490 | 1.978 | | | | |
| | 0ד.דו | 10 | 123.18 | 37.545 | 6.460 | 1.969 | 575 | 479 | 36.2 | ADJUST VALVE DOWN |
| | | 14 | 123.18 | 37.545 | 6.460 | 1.969 | 0.0 | | | PIZO TUBE FROZEN |
| | | 16 | 123.20 | 37.551 | 6.480 | 1.975 | | | | CLEAN OUT |
| | | 18 | 123.09 | 37.518 | 6.370 | 1.942 | | | | |
| | 14:50 | 20 | 122.91 | 37.463 | 6.190 | 1.887 | 530 | 442 | 33.4 | 20.5 IN. AT ORIFICE |
| | | 25 | 123.00 | 37.490 | 6.280 | 1.914 | | | | |
| | 15:01 | 31 | 123.00 | 37.490 | 6.280 | 1.914 | 527 | 439 | 33.2 | 18.5 IN. AT ORIFICE |
| | | 35 | 122.95 | 37.475 | 6.230 | 1.899 | 529 | 441 | 33.3 | 18 IN. AT ORIFICE |
| | 15:10 | 40 | 122.95 | 37.475 | 6.230 | 1.899 | 529 | 441 | 33.3 | |
| | 15:20 | 50 | 122.95 | 37.475 | 6.230 | 1.899 | 527 | 439 | 33.2 | |
| | 15:30 | 60 | 123.03 | 37.500 | 6.310 | 1.923 | 525 | 437 | 33.1 | 18 IN. AT ORIFICE |
| | 15:40 | 70 | 123.05 | 37.506 | 6.330 | 1.929 | 526 | 438 | 33.1 | |
| | 15:50 | 80 | 123.05 | 37.506 | 6.330 | 1.929 | 527 | 439 | 33.2 | 18 IN. AT ORIFICE |
| | 15:60 16:10 | 90 100 | 123.09 123.10 | 37.518 37.521 | 6.370 6.380 | 1.942 1.945 | 524 525 | 437 437 | 33.0 33.1 | 18 IN. AT ORIFICE |
| | 16:30 | 120 | 123.10 | 37.521 | 6.400 | 1.945 | 525 | 437 | 33.1 | 18 IN. AT ORIFICE |
| | 16:50 | 120 | 123.12 | 37.521 | 6.380 | 1.945 | 526 | 438 | 33.1 | |
| | 17:10 | 140 | 123.10 | | 6.380 | 1.945 | 526 | 438 | 33.1 | 18 IN. AT ORIFICE |
| | 17:30 | 180 | 123.15 | 37.536 | 6.430 | 1.960 | 020 | .00 | 0011 | PH 8.95, EC 1003, TDS 498 PPM |
| | 17:50 | 200 | 123.20 | 37.551 | 6.480 | 1.975 | 525 | 437 | 33.1 | SNOWING, COLD OUT, |
| | 18:40 | 250 | 123.15 | 37.536 | 6.430 | 1.960 | | - | | WATER NOT POOLING ANYWERE |
| | 19:30 | 300 | 123.28 | 37.576 | 6.560 | 1.999 | 526 | 438 | 33.1 | WATER SAMPLE TAKEN |
| | 20:20 | 350 | 123.31 | 37.585 | 6.590 | 2.009 | | | | TDS 2000 PPM, PH 8.86, EC 3999, TEMP 1.2°C |
| | 21:10 | 400 | 123.30 | 37.582 | 6.580 | 2.006 | 524 | 437 | 33.0 | |
| | 22:00 | 450 | 123.31 | 37.585 | 6.590 | 2.009 | | | | TDS 1343, PH 9.00, EC 2322, TEMP 0.2°C |
| | 22:50 | 500 | 123.30 | 37.582 | 6.580 | 2.006 | 525 | 437 | 33.1 | |
| | 23:40 | 550 | 123.31 | 37.585 | 6.590 | 2.009 | 525 | 437 | 33.1 | WATER SAMPLE #2 |
| 18-Jan | 0:30 | 600 | 123.32 | 37.588 | 6.600 | 2.012 | 524 | 437 | 33.0 | 18-Jan-12 |
| | 1:20 | 650 | 123.33 | 37.591 | 6.610 | 2.015 | 524 | 437 | 33.0 | TDS 1658, PH 9.12, EC 2638, TEMP. 0.9°C |
| | 2:10 3:00 | 700 750 | 123.35 123.40 | 37.597 37.612 | 6.630 | 2.021 2.036 | 526 | 420 | 22.4 | |
| | 3:00 3:50 | 750 800 | | | 6.680 | 2.036 | 526 525 | 438 437 | 33.1 33.1 | TDS 847, PH 9.05, EC 1942, TEMP 1.3°C |
| | 3:50 4:40 | 800 | 123.42 123.42 | 37.618 37.618 | 6.700 6.700 | 2.042 | 525 | 437 | 33.1 | TDS 836, PH 9.13, EC 1389, TEMP 2.7°C |
| | 5:30 | 900 | 123.42 | 37.609 | 6.670 | 2.042 | 525 | 437 | 33.0 | WATER SAMPLE #3 |
| | 6:20 | 900 950 | 123.39 | 37.618 | 6.700 | 2.033 | 524 | 437 | 33.0 | TDS 1061, PH 8.5, ECD 1681, TEMP 2.7°C |
| | 7:10 | 1000 | 123.38 | 37.606 | 6.660 | 2.042 | 524 | 438 | 33.1 | |
| | 8:00 | 1050 | 123.38 | 37.606 | 6.660 | 2.030 | 528 | 440 | 33.3 | 18 IN. AT ORIFICE - SNOWING |
| | 8:50 | 1100 | 123.37 | 37.603 | 6.650 | 2.027 | 526 | 438 | 33.1 | TDS 1051, PH 8.65, EC 1545, TEMP 2.4°C |
| | 9:40 | 1150 | 123.35 | 37.597 | 6.630 | 2.021 | 524 | 437 | 33.0 | WATER SAMPLE #3 |
| | 10:30 | 1200 | 123.36 | 37.600 | 6.640 | 2.024 | 525 | 437 | 33.1 | TDS 442, PH 7.55, EC 801, TEMP 0.7°C |
| | 11:20 | 1250 | 123.40 | | 6.680 | 2.036 | 526 | 438 | 33.1 | ALS WATER SAMPLES |
| | 12:10 | 1300 | 123.36 | 37.600 | 6.640 | 2.024 | 524 | 437 | 33.0 | TDS 80, PH 8.96, EC 225, TEMP 0.8 |
| | 13:00 | 1350 | 123.35 | 37.597 | 6.630 | 2.021 | 526 | 438 | 33.1 | 18 IN. AT ORIFICE |
| | 13:50 | 1400 | 123.42 | | 6.700 | 2.042 | 524 | 437 | 33.0 | |
| | 14:30 | 1440 | 123.36 | 37.600 | 6.640 | 2.024 | | | | |

TABLE D-2 SUMMARY OF MANUAL DATA DURING CONSTANT-RATE PUMPING TEST WITH TW12-1

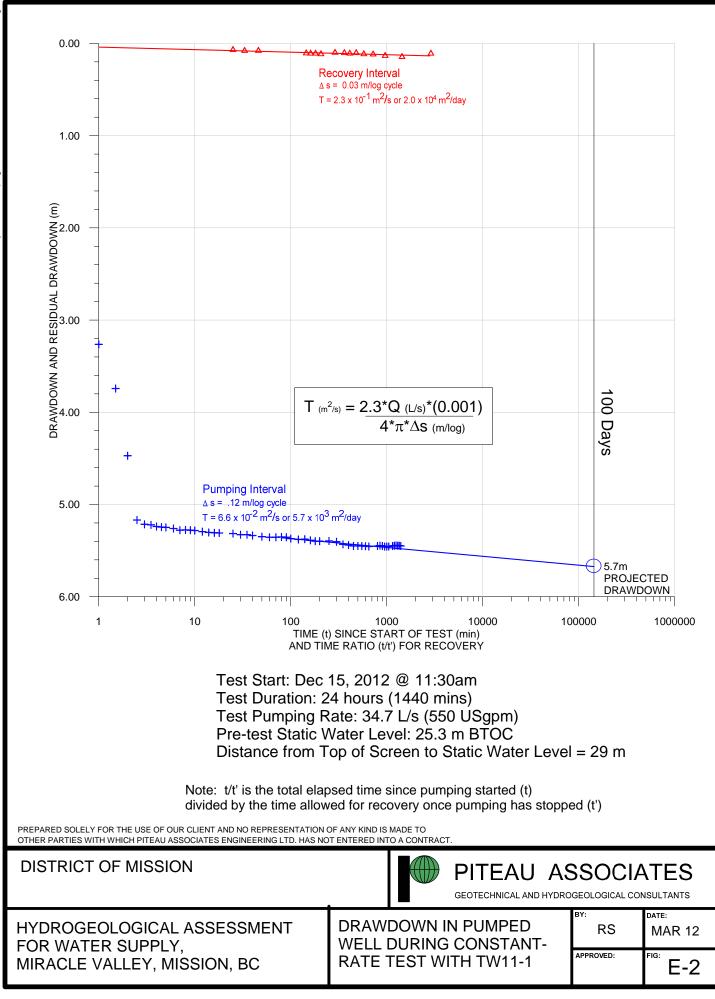
| | Clock | Elapsed | | Water | Levels | | _ | | | |
|-----------|---------|---------|--------|--------|--------|-------|--------------|--------|-------|--------------|
| Date 2012 | Time | Time, t | Depth | ьтос | Draw | down | Pumping Rate | | | Comments |
| | (h:m:s) | (min) | (ft) | (m) | (ft) | (m) | (USgpm) | (Igpm) | (L/s) | |
| | | | | | | | | | | RECOVERY |
| | | 1440.5 | 117.35 | 35.768 | 0.630 | 0.192 | 0 | 0 | 0.0 | |
| | | 1441 | 117.30 | 35.753 | 0.580 | 0.177 | | | | |
| | | 1441.5 | 117.29 | 35.750 | 0.570 | 0.174 | | | | |
| | | 1442 | 117.27 | 35.744 | 0.550 | 0.168 | | | | |
| | | 1442.5 | 117.25 | 35.738 | 0.530 | 0.162 | | | | |
| | | 1443 | 117.22 | 35.729 | 0.500 | 0.152 | | | | |
| | | 1443.5 | 117.21 | 35.726 | 0.490 | 0.149 | | | | |
| | | 1444 | 117.20 | 35.723 | 0.480 | 0.146 | | | | |
| | | 1444.5 | 117.19 | 35.720 | 0.470 | 0.143 | | | | |
| | 14:35 | 1445 | 117.18 | 35.716 | 0.460 | 0.140 | | | | |
| | | 1446 | 117.17 | 35.713 | 0.450 | 0.137 | | | | |
| | | 1447 | 117.15 | 35.707 | 0.430 | 0.131 | | | | |
| | | 1448 | 117.17 | 35.713 | 0.450 | 0.137 | | | | |
| | | 1449 | 117.16 | 35.710 | 0.440 | 0.134 | | | | |
| | 14:40 | 1450 | 117.15 | 35.707 | 0.430 | 0.131 | | | | |
| | | 1452 | 117.13 | 35.701 | 0.410 | 0.125 | | | | |
| | | 1454 | 117.12 | 35.698 | 0.400 | 0.122 | | | | |
| | | 1456 | 117.10 | 35.692 | 0.380 | 0.116 | | | | |
| | | 1458 | 117.09 | 35.689 | 0.370 | 0.113 | | | | |
| | 14:50 | 1460 | 117.08 | 35.686 | 0.360 | 0.110 | | | | |
| | | 1465 | 117.06 | 35.680 | 0.340 | 0.104 | | | | |
| | 15:00 | 1470 | 117.05 | 35.677 | 0.330 | 0.101 | | | | |
| | | 1475 | 117.04 | 35.674 | 0.320 | 0.098 | | | | |
| | 15:10 | 1480 | 117.02 | 35.668 | 0.300 | 0.091 | | | | |
| | 15:20 | 1490 | 117.00 | 35.662 | 0.280 | 0.085 | | | | |
| | 15:30 | 1500 | 116.99 | 35.659 | 0.270 | 0.082 | | | | |
| | 15:40 | 1510 | 116.97 | 35.652 | 0.250 | 0.076 | | | | |
| | 15:55 | 1525 | 116.96 | 35.649 | 0.240 | 0.073 | | | | |
| | 16:10 | 1540 | 116.95 | 35.646 | 0.230 | 0.070 | | | | |
| | 16:30 | 1560 | 116.95 | 35.646 | 0.230 | 0.070 | | | | END RECOVERY |

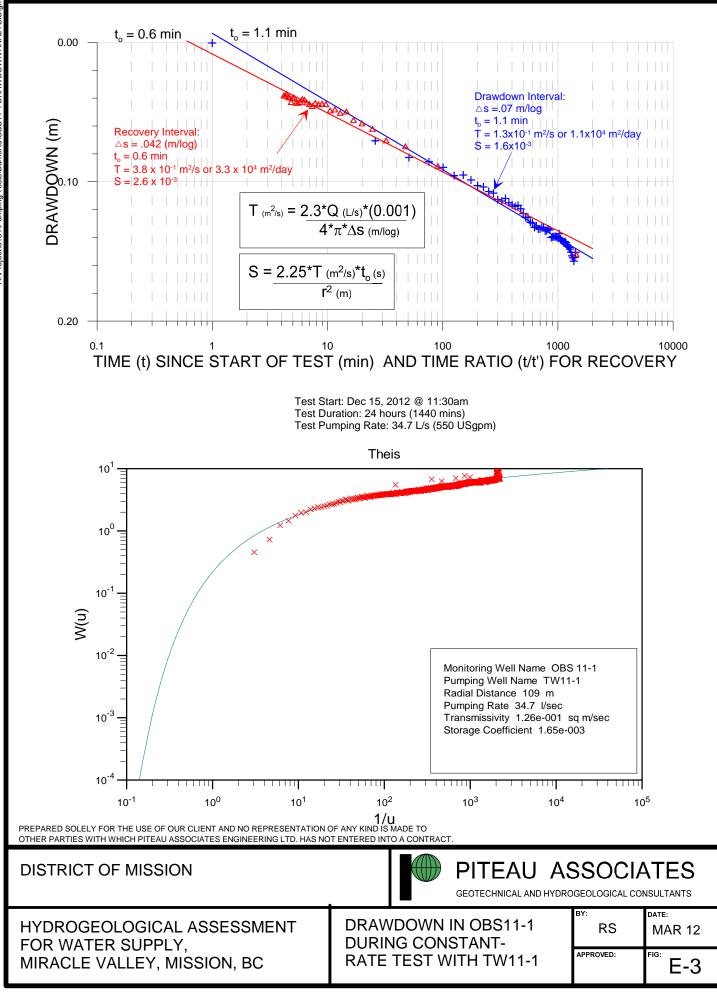
APPENDIX E

GRAPHICAL ANALYSIS OF VARIABLE- AND CONSTANT-RATE TESTS WITH TW11-1



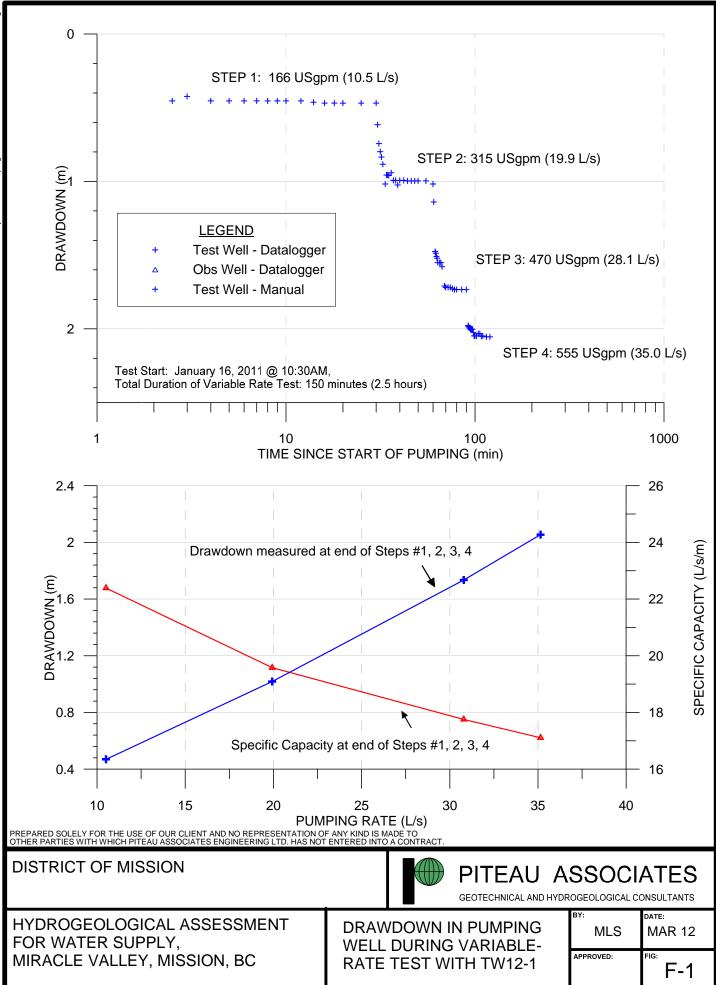
H:\Project\3131\Pumping Tests\BurnsRd\TW11-1 STEP TEST.gr

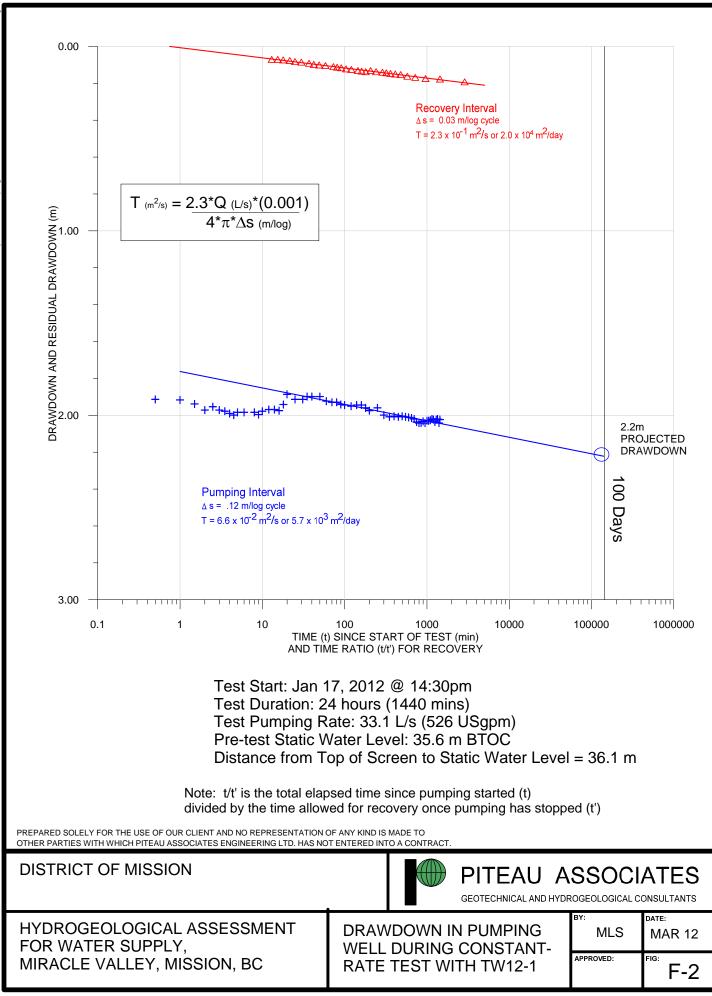


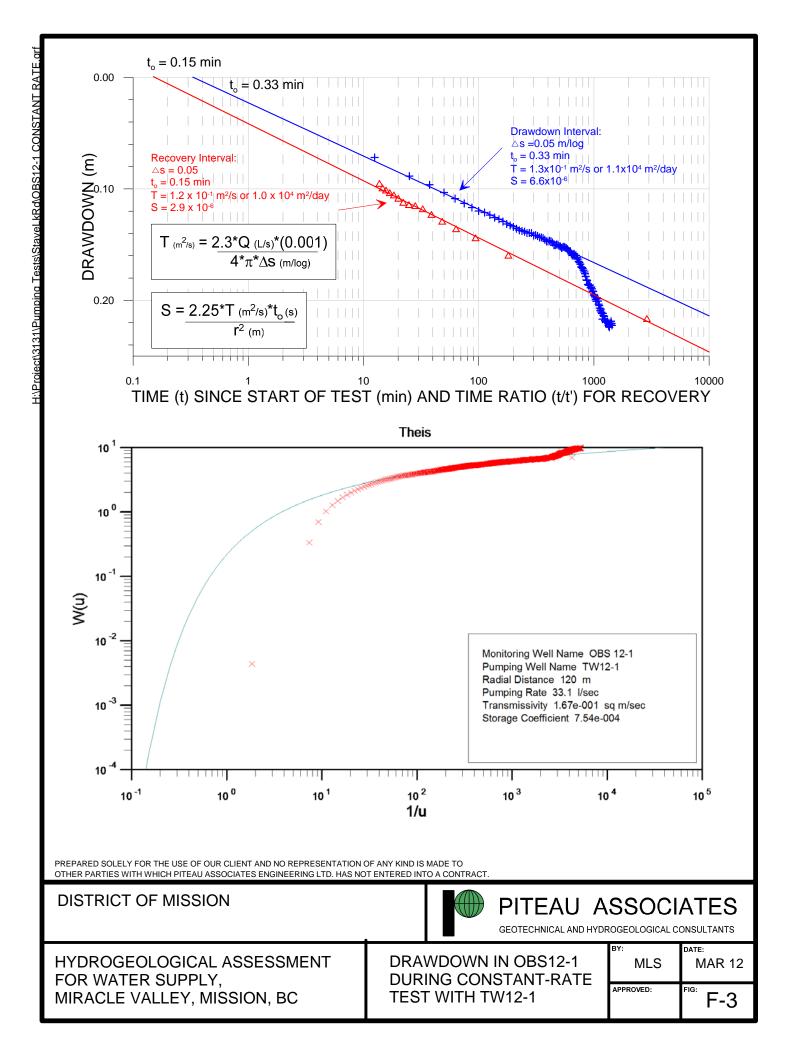


APPENDIX F

GRAPHICAL ANALYSIS OF VARIABLE- AND CONSTANT-RATE TESTS WITH TW12-1







APPENDIX G

LABORATORY ANALYSES REPORTS



PITEAU ASSOC. ENGINEERING LTD. ATTN: Kathy Tixier # 215 - 260 West Esplanade North Vancouver BC V7M 3G7 Date Received:15-DEC-11Report Date:23-DEC-11 15:09 (MT)Version:FINAL

Client Phone: 604-986-8551

Certificate of Analysis

Lab Work Order #: L1096345

Project P.O. #: Job Reference: C of C Numbers: Legal Site Desc: NOT SUBMITTED 3131

Mark

Brent Mack Account Manager

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ALS ENVIRONMENTAL ANALYTICAL REPORT

L1096345 CONTD.... PAGE 2 of 4 23-DEC-11 15:09 (MT) Version: FINAL

| | Sample ID Description Sampled Date Sampled Time Client ID | L1096345-1 WATER 14-DEC-11 11:45 LANG RESIDENCE | | |
|-------------------------|---|---|--|--|
| Grouping | Analyte | | | |
| WATER | | | | |
| Physical Tests | Colour, True (CU) | <5.0 | | |
| | Conductivity (uS/cm) | 58.8 | | |
| | Hardness (as CaCO3) (mg/L) | 23.8 | | |
| | рН (рН) | 7.51 | | |
| | Total Dissolved Solids (mg/L) | 44 | | |
| | Turbidity (NTU) | 1.12 | | |
| Anions and Nutrients | Alkalinity, Total (as CaCO3) (mg/L) | 23.3 | | |
| | Chloride (Cl) (mg/L) | 1.21 | | |
| | Fluoride (F) (mg/L) | <0.020 | | |
| | Nitrate (as N) (mg/L) | 0.613 | | |
| | Nitrite (as N) (mg/L) | <0.0010 | | |
| | Sulfate (SO4) (mg/L) | 2.39 | | |
| Total Metals | Aluminum (Al)-Total (mg/L) | <0.010 | | |
| | Antimony (Sb)-Total (mg/L) | <0.00050 | | |
| | Arsenic (As)-Total (mg/L) | 0.00010 | | |
| | Barium (Ba)-Total (mg/L) | <0.020 | | |
| | Boron (B)-Total (mg/L) | <0.10 | | |
| | Cadmium (Cd)-Total (mg/L) | <0.00020 | | |
| | Calcium (Ca)-Total (mg/L) | 7.76 | | |
| | Chromium (Cr)-Total (mg/L) | <0.0020 | | |
| | Copper (Cu)-Total (mg/L) | 0.0064 | | |
| | Iron (Fe)-Total (mg/L) | 0.197 | | |
| | Lead (Pb)-Total (mg/L) | <0.00050 | | |
| | Magnesium (Mg)-Total (mg/L) | 1.07 | | |
| | Manganese (Mn)-Total (mg/L) | 0.0089 | | |
| | Mercury (Hg)-Total (mg/L) | <0.00020 | | |
| | Potassium (K)-Total (mg/L) | 0.38 | | |
| | Selenium (Se)-Total (mg/L) | <0.0010 | | |
| | Sodium (Na)-Total (mg/L) | <2.0 | | |
| | Uranium (U)-Total (mg/L) | <0.00010 | | |
| | Zinc (Zn)-Total (mg/L) | <0.050 | | |
| | | | | |

* Please refer to the Reference Information section for an explanation of any qualifiers detected.

Reference Information

| QC Type Description | | Parameter | Qualifier | Applies to Sample Number(s) |
|--|-------------------------------------|--|--|--|
| Duplicate | | Chloride (Cl) | DLM | L1096345-1 |
| Duplicate | | Fluoride (F) | DLM | L1096345-1 L1096345-1 |
| Duplicate | | Nitrite (as N) | DLM | L1096345-1 |
| Matrix Spike | | Mercury (Hg)-Total | MS-B | L1096345-1 |
| Qualifiers for Individu | al Daramatara | , , , , | | 210000101 |
| Qualifier Descri | | Listea: | | |
| | | ted For Sample Matrix Effects | | |
| | | could not be accurately calculated | due to high analyte | hackground in sample |
| | . , | | | |
| est Method Reference | | Tart Daganintian | | Mathed Deference** |
| ALS Test Code | Matrix | Test Description | | Method Reference** |
| ALK-COL-VA | Water | Alkalinity by Colourimetric (Auto | , | APHA 310.2 |
| colourimetric method. | out using proce | edures adapted from EPA Method 3 | 310.2 "Alkalinity". Tot | tal Alkalinity is determined using the methyl orange |
| NIONS-CL-IC-VA | Water | Chloride by Ion Chromatography | | APHA 4110 B. |
| | | edures adapted from APHA Method Determination of Inorganic Anions | | atography with Chemical Suppression of Eluent ohy". |
| NIONS-F-IC-VA | Water | Fluoride by Ion Chromatography | / | APHA 4110 B. |
| | | edures adapted from APHA Method Determination of Inorganic Anions | | atography with Chemical Suppression of Eluent ohy". |
| NIONS-NO2-IC-VA | Water | Nitrite in Water by Ion Chromato | ography | EPA 300.0 |
| This analysis is carried detected by UV absorba | | edures adapted from EPA Method 3 | 300.0 "Determination | of Inorganic Anions by Ion Chromatography". Nitrite is |
| NIONS-NO3-IC-VA | Water | Nitrate in Water by Ion Chromat | ography | EPA 300.0 |
| This analysis is carried detected by UV absorba | | edures adapted from EPA Method 3 | 300.0 "Determination | of Inorganic Anions by Ion Chromatography". Nitrate is |
| NIONS-SO4-IC-VA | Water | Sulfate by Ion Chromatography | | APHA 4110 B. |
| | | edures adapted from APHA Method Determination of Inorganic Anions | | atography with Chemical Suppression of Eluent oby". |
| OLOUR-TRUE-VA | Water | Colour (True) by Spectrometer | | BCMOE Colour Single Wavelength |
| is determined by filterin method. Aparent Colou | g a sample thro ir is determined | ough a 0.45 micron membrane filter | followed by analysis | anual "Colour- Single Wavelength." Colour (True Colour s of the filtrate using the platinum-cobalt colourimetric t. Unless otherwise indicated, reported colour results |
| EC-PCT-VA | Water | Conductivity (Automated) | | APHA 2510 Auto. Conduc. |
| This analysis is carried electrode. | out using proce | edures adapted from APHA Method | 2510 "Conductivity" | . Conductivity is determined using a conductivity |
| IARDNESS-CALC-VA | Water | Hardness | | APHA 2340B |
| | | ess) is calculated from the sum of C ncentrations are preferentially used | | ium concentrations, expressed in CaCO3 equivalents. Iculation. |
| IG-TOT-CVAFS-VA | Water | Total Mercury in Water by CVAF | -s | EPA 245.7 |
| American Public Health States Environmental P | Association, a rotection Agen | nd with procedures adapted from " cy (EPA). The procedure involves | Test Methods for Eva a cold-oxidation of th | ation of Water and Wastewater" published by the aluating Solid Waste" SW-846 published by the United he acidified sample using bromine monochloride prior to c fluorescence spectrophotometry (EPA Method 245.7) |
| IET-TOT-ICP-VA | Water | Total Metals in Water by ICPOE | S | EPA SW-846 3005A/6010B |
| American Public Health States Environmental P | Association, a rotection Agen | nd with procedures adapted from " cy (EPA). The procedures may inv | Test Methods for Eva olve preliminary sam | ation of Water and Wastewater" published by the aluating Solid Waste" SW-846 published by the United aple treatment by acid digestion, using either hotblock o a - optical emission spectrophotometry (EPA Method |
| IET-TOT-LOW-MS-VA | Water | Total Metals in Water by ICPMS | (Low) | EPA SW-846 3005A/6020A |
| | | | | ation of Water and Wastewater" published by the aluating Solid Waste" SW-846 published by the United |

American Public Health Association, and with procedures adapted from "Test Methods for Evaluating Solid Waste" SW-846 published by the United States Environmental Protection Agency (EPA). The procedures may involve preliminary sample treatment by acid digestion, using either hotblock or microwave oven, or filtration (EPA Method 3005A). Instrumental analysis is by inductively coupled plasma - mass spectrometry (EPA Method 6020A).

Reference Information

L1096345 CONTD.... PAGE 4 of 4 23-DEC-11 15:09 (MT) Version: FINAL

| PH-PCT-VA | Water | pH by Meter (Automated) | APHA 4500-H "pH Value" |
|--|--|--|--|
| This analysis is carried ou electrode | ut using proc | edures adapted from APHA Method 4500-H "pl | H Value". The pH is determined in the laboratory using a pH |
| It is recommended that th | is analysis b | e conducted in the field. | |
| PH-PCT-VA | Water | pH by Meter (Automated) | APHA 4500-H pH Value |
| This analysis is carried ou electrode | ut using proc | edures adapted from APHA Method 4500-H "pl | H Value". The pH is determined in the laboratory using a pH |
| It is recommended that th | is analysis b | e conducted in the field. | |
| TDS-VA | Water | Total Dissolved Solids by Gravimetric | APHA 2540 C - GRAVIMETRIC |
| | | | ds". Solids are determined gravimetrically. Total Dissolved Solids ined by evaporating the filtrate to dryness at 180 degrees celsius. |
| TURBIDITY-VA | Water | Turbidity by Meter | APHA 2130 "Turbidity" |
| This analysis is carried ou | ut using proc | edures adapted from APHA Method 2130 "Turk | idity". Turbidity is determined by the nephelometric method. |
| TURBIDITY-VA | Water | Turbidity by Meter | APHA 2130 Turbidity |
| This analysis is carried ou | ut using proc | edures adapted from APHA Method 2130 "Turk | oidity". Turbidity is determined by the nephelometric method. |
| ** ALS test methods may inc | corporate mo | odifications from specified reference methods to |) improve performance. |
| The last two letters of the a | above test co | ode(s) indicate the laboratory that performed an | alytical analysis for that test. Refer to the list below: |
| Laboratory Definition Co | de Labo | ratory Location | |
| VA | ALS E | ENVIRONMENTAL - VANCOUVER, BC, CANA | DA |
| Chain of Custody Number | s: | | |
| applicable tests, surrogates mg/kg - milligrams per kilog mg/kg wwt - milligrams per | nat is similar s are added gram based o kilogram ba | to samples prior to analysis as a check on reco | not occur naturally in environmental samples. For very. |

D.L. - The reported Detection Limit, also known as the Limit of Reporting (LOR).

N/A - Result not available. Refer to qualifier code and definition for explanation.

Test results reported relate only to the samples as received by the laboratory. UNLESS OTHERWISE STATED, ALL SAMPLES WERE RECEIVED IN ACCEPTABLE CONDITION. Analytical results in unsigned test reports with the DRAFT watermark are subject to change, pending final QC review.

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| Contact: | Kathy Tixier | | | PDF | Excel | Digital | Fax | _ | | | | | | | ntact ALS t | o Confirm | TAT |
| Address: | 215-260 West Espla | inade | | Email 1: | ktixier@piteau. | | | - | | _ | | | | | ontact ALS | | |
| | | | | Email 2: | | | | _ | | | _ | | | | o Confirm | | |
| Phone: | 604-986-8551 | Fax: 6 | 604-985-7286 | Email 3: | · | | | | | | | | alysis R | | | | |
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PITEAU ASSOC. ENGINEERING LTD. ATTN: Kathy Tixier # 215 - 260 West Esplanade North Vancouver BC V7M 3G7 Date Received: 08-DEC-11 Report Date: 14-DEC-11 15:38 (MT) Version: FINAL

Client Phone: 604-986-8551

Certificate of Analysis

Lab Work Order #: L1093778

Project P.O. #: Job Reference: C of C Numbers: Legal Site Desc: NOT SUBMITTED 3131 10-195225

Mark

Brent Mack Account Manager

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ALS ENVIRONMENTAL ANALYTICAL REPORT

L1093778 CONTD.... PAGE 2 of 5 14-DEC-11 15:38 (MT) Version: FINAL

| | Sample ID Description Sampled Date Sampled Time Client ID | L1093778-1 WATER 08-DEC-11 12:00 W255 | |
|-------------------------|---|---|--|
| Grouping | Analyte | - | |
| WATER | | | |
| Physical Tests | Hardness (as CaCO3) (mg/L) | 75.7 | |
| Total Metals | Aluminum (Al)-Total (mg/L) | 191 | |
| | Antimony (Sb)-Total (mg/L) | DLA <0.010 | |
| | Arsenic (As)-Total (mg/L) | 0.092 | |
| | Barium (Ba)-Total (mg/L) | 1.27 | |
| | Beryllium (Be)-Total (mg/L) | <0.0050 | |
| | Boron (B)-Total (mg/L) | <0.10 | |
| | Cadmium (Cd)-Total (mg/L) | 0.0058 | |
| | Calcium (Ca)-Total (mg/L) | 153 | |
| | Chromium (Cr)-Total (mg/L) | 0.388 | |
| | Cobalt (Co)-Total (mg/L) | 0.190 | |
| | Copper (Cu)-Total (mg/L) | 0.735 | |
| | Iron (Fe)-Total (mg/L) | 300 | |
| | Lead (Pb)-Total (mg/L) | 0.125 | |
| | Lithium (Li)-Total (mg/L) | 0.118 | |
| | Magnesium (Mg)-Total (mg/L) | 129 | |
| | Manganese (Mn)-Total (mg/L) | 6.62 | |
| | Mercury (Hg)-Total (mg/L) | 0.00046 | |
| | Molybdenum (Mo)-Total (mg/L) | 0.029 | |
| | Nickel (Ni)-Total (mg/L) | 0.62 | |
| | Selenium (Se)-Total (mg/L) | OLA <0.020 | |
| | Silver (Ag)-Total (mg/L) | 0.0016 | |
| | Sodium (Na)-Total (mg/L) | 19.7 | |
| | Thallium (TI)-Total (mg/L) | DLA <0.0040 | |
| | Titanium (Ti)-Total (mg/L) | 10.2 | |
| | Uranium (U)-Total (mg/L) | 0.0067 | |
| | Vanadium (V)-Total (mg/L) | 0.528 | |
| | Zinc (Zn)-Total (mg/L) | 0.663 | |
| Dissolved Metals | Aluminum (Al)-Dissolved (mg/L) | 0.174 | |
| | Antimony (Sb)-Dissolved (mg/L) | 0.00705 | |
| | Arsenic (As)-Dissolved (mg/L) | 0.0051 | |
| | Barium (Ba)-Dissolved (mg/L) | 0.021 | |
| | Beryllium (Be)-Dissolved (mg/L) | <0.0050 | |
| | Boron (B)-Dissolved (mg/L) | <0.10 | |
| | Cadmium (Cd)-Dissolved (mg/L) | <0.000050 | |
| | Calcium (Ca)-Dissolved (mg/L) | 23.0 | |
| | Chromium (Cr)-Dissolved (mg/L) | <0.00050 | |

* Please refer to the Reference Information section for an explanation of any qualifiers detected.

ALS ENVIRONMENTAL ANALYTICAL REPORT

L1093778 CONTD.... PAGE 3 of 5 14-DEC-11 15:38 (MT) Version: FINAL

| | Sample ID Description Sampled Date Sampled Time Client ID | L1093778-1 WATER 08-DEC-11 12:00 W255 | | |
|------------------|---|---|--|--|
| Grouping | Analyte | | | |
| WATER | | | | |
| Dissolved Metals | Cobalt (Co)-Dissolved (mg/L) | <0.00050 | | |
| | Copper (Cu)-Dissolved (mg/L) | 0.0010 | | |
| | Iron (Fe)-Dissolved (mg/L) | 0.105 | | |
| | Lead (Pb)-Dissolved (mg/L) | <0.0010 | | |
| | Lithium (Li)-Dissolved (mg/L) | <0.050 | | |
| | Magnesium (Mg)-Dissolved (mg/L) | 4.41 | | |
| | Manganese (Mn)-Dissolved (mg/L) | 0.039 | | |
| | Mercury (Hg)-Dissolved (mg/L) | <0.00020 | | |
| | Molybdenum (Mo)-Dissolved (mg/L) | 0.0142 | | |
| | Nickel (Ni)-Dissolved (mg/L) | 0.0075 | | |
| | Selenium (Se)-Dissolved (mg/L) | 0.0012 | | |
| | Silver (Ag)-Dissolved (mg/L) | <0.000050 | | |
| | Sodium (Na)-Dissolved (mg/L) | 6.1 | | |
| | Thallium (TI)-Dissolved (mg/L) | <0.00020 | | |
| | Titanium (Ti)-Dissolved (mg/L) | <0.050 | | |
| | Uranium (U)-Dissolved (mg/L) | 0.00054 | | |
| | Vanadium (V)-Dissolved (mg/L) | <0.030 | | |
| | Zinc (Zn)-Dissolved (mg/L) | <0.0050 | | |
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* Please refer to the Reference Information section for an explanation of any qualifiers detected.

Reference Information

Qualifiers for Individual Parameters Listed:

| Qualifier | Description | | |
|---|--|--|--|
| DLA | • | sted For required dilution | |
| est Method R | eferences: | | |
| ALS Test Code | Matrix | Test Description | Method Reference** |
| HARDNESS-CA | LC-VA Water | Hardness | APHA 2340B |
| | | ess) is calculated from the sum of oncentrations are preferentially use | Calcium and Magnesium concentrations, expressed in CaCO3 equivalents. |
| HG-DIS-CVAFS- | VA Water | Dissolved Mercury in Water by | CVAFS EPA SW-846 3005A & EPA 245.7 |
| American Publi States Environi involves a cold- | c Health Association, a nental Protection Ager oxidation of the acidifie | and with procedures adapted from ncy (EPA). The procedures may ir | ethods for the Examination of Water and Wastewater" published by the "Test Methods for Evaluating Solid Waste" SW-846 published by the United wolve preliminary sample treatment by filtration (EPA Method 3005A) and oride prior to reduction of the sample with stannous chloride. Instrumental Method 245.7). |
| HG-TOT-CVAFS | -VA Water | Total Mercury in Water by CVA | AFS EPA 245.7 |
| American Publi States Environi | c Health Association, a nental Protection Ager | and with procedures adapted from ncy (EPA). The procedure involves | ethods for the Examination of Water and Wastewater" published by the "Test Methods for Evaluating Solid Waste" SW-846 published by the United s a cold-oxidation of the acidified sample using bromine monochloride prior to s by cold vapour atomic fluorescence spectrophotometry (EPA Method 245.7). |
| MET-DIS-ICP-V | Water | Dissolved Metals in Water by I | CPOES EPA SW-846 3005A/6010B |
| American Publi States Environi | c Health Association, a | and with procedures adapted from ncy (EPA). The procedure involves | ethods for the Examination of Water and Wastewater" published by the "Test Methods for Evaluating Solid Waste" SW-846 published by the United s filtration (EPA Method 3005A) and analysis by inductively coupled plasma - |
| MET-DIS-LOW-I | IS-VA Water | Dissolved Metals in Water by I | CPMS(Low) EPA SW-846 3005A/6020A |
| American Publi States Environi | c Health Association, a nental Protection Ager | and with procedures adapted from | ethods for the Examination of Water and Wastewater" published by the "Test Methods for Evaluating Solid Waste" SW-846 published by the United es preliminary sample treatment by filtration (EPA Method 3005A). etry (EPA Method 6020A). |
| MET-TOT-ICP-V | A Water | Total Metals in Water by ICPO | ES EPA SW-846 3005A/6010B |
| American Publi States Environi | c Health Association, a nental Protection Ager | and with procedures adapted from ncy (EPA). The procedures may ir | ethods for the Examination of Water and Wastewater" published by the "Test Methods for Evaluating Solid Waste" SW-846 published by the United avolve preliminary sample treatment by acid digestion, using either hotblock or ctively coupled plasma - optical emission spectrophotometry (EPA Method |
| MET-TOT-LOW- | MS-VA Water | Total Metals in Water by ICPN | IS(Low) EPA SW-846 3005A/6020A |
| American Publi States Environ | c Health Association, a nental Protection Ager | and with procedures adapted from ncy (EPA). The procedures may ir | ethods for the Examination of Water and Wastewater" published by the "Test Methods for Evaluating Solid Waste" SW-846 published by the United wolve preliminary sample treatment by acid digestion, using either hotblock or is is by inductively coupled plasma - mass spectrometry (EPA Method 6020A) |
| * ALS test metho | ds may incorporate mo | odifications from specified reference | e methods to improve performance. |
| The last two lette | ers of the above test co | ode(s) indicate the laboratory that p | performed analytical analysis for that test. Refer to the list below: |
| Laboratory Defi | nition Code Labo | ratory Location | |
| VA | ALS I | ENVIRONMENTAL - VANCOUVE | R, BC, CANADA |
| Chain of Custod | / Numbers: | | |
| | | | |

Reference Information

GLOSSARY OF REPORT TERMS

Surrogate - A compound that is similar in behaviour to target analyte(s), but that does not occur naturally in environmental samples. For applicable tests, surrogates are added to samples prior to analysis as a check on recovery. mg/kg - milligrams per kilogram based on dry weight of sample.

mg/kg wwt - milligrams per kilogram based on wet weight of sample.

mg/kg lwt - milligrams per kilogram based on lipid-adjusted weight of sample.

mg/L - milligrams per litre.

< - Less than.

D.L. - The reported Detection Limit, also known as the Limit of Reporting (LOR).

N/A - Result not available. Refer to qualifier code and definition for explanation.

Test results reported relate only to the samples as received by the laboratory.

UNLESS OTHERWISE STATED, ALL SAMPLES WERE RECEIVED IN ACCEPTABLE CONDITION.

Analytical results in unsigned test reports with the DRAFT watermark are subject to change, pending final QC review.

10-195225

Page _____ of _____

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| Contact: Kathy TIXtier | Select: PD | <u>r Segovia</u> | | Fax | | | | | | | e - Contact | _ | | r | |
| Address: 215 - 360 Esplana le ave No Vancouver BC | Email 1; Email 2: | | C pite | | | | | | | | S to confirm | | | - | |
| Phone: GOU 996-8551 Fax: | | <u>N T IXTICI</u> | <u>e pite</u> | av com | | | | | | sis Rec | | | <u> </u> | | |
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| Phone: Fax: | Quote #: | | | | W I | | | | | | | | | | tain |
| Lab Work Order # (lab use only) U10913778 | ALS Contact: | Brent M. | Sampler: Ri | Cardo | olved N | | | | | | | | | | Number of Containers |
| Sample # (This description will appear on the report) | | Date (dd-mmm-yy) | Time (hh:mm) | Sample Type | Diss | | | | | | | | | | Numbe |
| W255 | | 08-DEC-11 | 12:00 | Water | ∇ | | | | | \square | | | \square | | |
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| REFER TO BACK PAGE FOR ALS LOCATIONS AND SAMPLING INFORMATION | UN | | WHITE - LAB | ORATORY COPY | YELL | .uw - C | LIENT C | UP1 | | | | GEN | IF 18.01 | ront | |



PITEAU ASSOC. ENGINEERING LTD. ATTN: Kathy Tixier # 215 - 260 West Esplanade North Vancouver BC V7M 3G7 Date Received: 16-DEC-11 Report Date: 28-DEC-11 15:26 (MT) Version: FINAL

Client Phone: 604-986-8551

Certificate of Analysis

Lab Work Order #: L1096762

Project P.O. #: Job Reference: C of C Numbers: Legal Site Desc: NOT SUBMITTED 3131 10-196045

Mark

Brent Mack Account Manager

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ALS ENVIRONMENTAL ANALYTICAL REPORT

L1096762 CONTD.... PAGE 2 of 6 28-DEC-11 15:26 (MT) Version: FINAL

| | Sample ID Description Sampled Date Sampled Time Client ID | L1096762-1 WATER 16-DEC-11 12:00 MV2011-1 | | |
|--------------------------|---|---|--|--|
| Grouping | Analyte | | | |
| WATER | | | | |
| Physical Tests | UV Absorbance (254 nm) (Abs/cm-1) | <0.0010 | | |
| | Colour, True (CU) | <5.0 | | |
| | Conductivity (uS/cm) | 122 | | |
| | Hardness (as CaCO3) (mg/L) | 55.6 | | |
| | рН (рН) | 7.72 | | |
| | Total Dissolved Solids (mg/L) | 76 | | |
| | Turbidity (NTU) | <0.10 | | |
| Anions and Nutrients | Alkalinity, Bicarbonate (as CaCO3) (mg/L) | 49.5 | | |
| | Alkalinity, Carbonate (as CaCO3) (mg/L) | <2.0 | | |
| | Alkalinity, Hydroxide (as CaCO3) (mg/L) | <2.0 | | |
| | Alkalinity, Total (as CaCO3) (mg/L) | 49.5 | | |
| | Chloride (Cl) (mg/L) | 0.95 | | |
| | Fluoride (F) (mg/L) | 0.031 | | |
| | Nitrate (as N) (mg/L) | 0.204 | | |
| | Nitrite (as N) (mg/L) | <0.0010 | | |
| | Sulfate (SO4) (mg/L) | 9.41 | | |
| | Sulphide as S (mg/L) | <0.020 | | |
| Bacteriological Tests | E. coli (MPN/100mL) | <1 | | |
| | Coliform Bacteria - Total (MPN/100mL) | <1 | | |
| Fotal Metals | Aluminum (Al)-Total (mg/L) | <0.010 | | |
| | Antimony (Sb)-Total (mg/L) | <0.00050 | | |
| | Arsenic (As)-Total (mg/L) | 0.00090 | | |
| | Barium (Ba)-Total (mg/L) | <0.020 | | |
| | Boron (B)-Total (mg/L) | <0.10 | | |
| | Cadmium (Cd)-Total (mg/L) | <0.00020 | | |
| | Calcium (Ca)-Total (mg/L) | 16.1 | | |
| | Chromium (Cr)-Total (mg/L) | <0.0020 | | |
| | Copper (Cu)-Total (mg/L) | <0.0010 | | |
| | Iron (Fe)-Total (mg/L) | <0.030 | | |
| | Lead (Pb)-Total (mg/L) | <0.00050 | | |
| | Magnesium (Mg)-Total (mg/L) | 3.46 | | |
| | Manganese (Mn)-Total (mg/L) | <0.0020 | | |
| | Mercury (Hg)-Total (mg/L) | <0.00020 | | |
| | Potassium (K)-Total (mg/L) | 0.83 | | |
| | Selenium (Se)-Total (mg/L) | <0.0010 | | |
| | Sodium (Na)-Total (mg/L) | 2.5 | | |

ALS ENVIRONMENTAL ANALYTICAL REPORT

L1096762 CONTD.... PAGE 3 of 6 28-DEC-11 15:26 (MT) Version: FINAL

| | Sample ID Description Sampled Date Sampled Time Client ID | L1096762-1 WATER 16-DEC-11 12:00 MV2011-1 | | |
|-------------------------------|---|---|--|--|
| Grouping | Analyte | | | |
| WATER | | | | |
| Total Metals | Uranium (U)-Total (mg/L) | <0.00010 | | |
| | Zinc (Zn)-Total (mg/L) | <0.050 | | |
| Dissolved Metals | Aluminum (Al)-Dissolved (mg/L) | <0.010 | | |
| | Antimony (Sb)-Dissolved (mg/L) | <0.00050 | | |
| | Arsenic (As)-Dissolved (mg/L) | 0.00091 | | |
| | Barium (Ba)-Dissolved (mg/L) | <0.020 | | |
| | Boron (B)-Dissolved (mg/L) | <0.10 | | |
| | Cadmium (Cd)-Dissolved (mg/L) | <0.00020 | | |
| | Calcium (Ca)-Dissolved (mg/L) | 16.5 | | |
| | Chromium (Cr)-Dissolved (mg/L) | <0.0020 | | |
| | Copper (Cu)-Dissolved (mg/L) | <0.0010 | | |
| | Iron (Fe)-Dissolved (mg/L) | <0.030 | | |
| | Lead (Pb)-Dissolved (mg/L) | <0.00050 | | |
| | Magnesium (Mg)-Dissolved (mg/L) | 3.50 | | |
| | Manganese (Mn)-Dissolved (mg/L) | <0.0020 | | |
| | Mercury (Hg)-Dissolved (mg/L) | <0.00020 | | |
| | Potassium (K)-Dissolved (mg/L) | 0.81 | | |
| | Selenium (Se)-Dissolved (mg/L) | <0.0010 | | |
| | Sodium (Na)-Dissolved (mg/L) | 2.5 | | |
| | Uranium (U)-Dissolved (mg/L) | <0.00010 | | |
| | Zinc (Zn)-Dissolved (mg/L) | <0.050 | | |
| Volatile Organic Compounds | Benzene (mg/L) | <0.00050 | | |
| | Ethylbenzene (mg/L) | <0.00050 | | |
| | Methyl t-butyl ether (MTBE) (mg/L) | <0.00050 | | |
| | Toluene (mg/L) | <0.00050 | | |
| | ortho-Xylene (mg/L) | <0.00050 | | |
| | meta- & para-Xylene (mg/L) | <0.00050 | | |
| | Xylenes (mg/L) | <0.00075 | | |
| | Surrogate: 4-Bromofluorobenzene (SS) (%) | 102.7 | | |
| | Surrogate: 1,4-Difluorobenzene (SS) (%) | 99.5 | | |
| Hydrocarbons | EPH10-19 (mg/L) | <0.25 | | |
| | EPH19-32 (mg/L) | <0.25 | | |
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Reference Information

Test Method References:

| Lest Method References | | Toot Decerintian | Mothod Poforonco** |
|---|--|--|---|
| ALS Test Code | Matrix | Test Description | Method Reference** |
| ALK-SCR-VA | Water | Alkalinity by colour or titration | EPA 310.2 OR APHA 2320 |
| colourimetric method. OR | 01 | | Total Alkalinity is determined using the methyl orange |
| | | ures adapted from APHA Method 2320 "Alkalinity". and hydroxide alkalinity are calculated from pheno | . Total alkalinity is determined by potentiometric titration to a olphthalein alkalinity and total alkalinity values. |
| ANIONS-CL-IC-VA | Water | Chloride by Ion Chromatography | APHA 4110 B. |
| | | ures adapted from APHA Method 4110 B. "Ion Chreater and the etermination of Inorganic Anions by Ion Chromatog | omatography with Chemical Suppression of Eluent graphy". |
| ANIONS-F-IC-VA | Water | Fluoride by Ion Chromatography | APHA 4110 B. |
| | | ures adapted from APHA Method 4110 B. "Ion Chroterer adapted from APHA Method 4110 B. "Ion Chromatog | romatography with Chemical Suppression of Eluent graphy". |
| ANIONS-NO2-IC-VA | Water | Nitrite in Water by Ion Chromatography | EPA 300.0 |
| This analysis is carried out detected by UV absorbance | 01 | ures adapted from EPA Method 300.0 "Determinati | ion of Inorganic Anions by Ion Chromatography". Nitrite is |
| ANIONS-NO3-IC-VA | Water | Nitrate in Water by Ion Chromatography | EPA 300.0 |
| This analysis is carried out detected by UV absorbance | 01 | ures adapted from EPA Method 300.0 "Determinati | ion of Inorganic Anions by Ion Chromatography". Nitrate is |
| ANIONS-SO4-IC-VA | Water | Sulfate by Ion Chromatography | APHA 4110 B. |
| | | ures adapted from APHA Method 4110 B. "Ion Chro etermination of Inorganic Anions by Ion Chromatog | omatography with Chemical Suppression of Eluent graphy". |
| COLOUR-TRUE-VA | Water | Colour (True) by Spectrometer | BCMOE Colour Single Wavelength |
| is determined by filtering a | sample throug determined w | gh a 0.45 micron membrane filter followed by analy vithout prior sample filtration. Colour is pH depend | Manual "Colour- Single Wavelength." Colour (True Colour) rsis of the filtrate using the platinum-cobalt colourimetric ent. Unless otherwise indicated, reported colour results |
| EC-PCT-VA | Water | Conductivity (Automated) | APHA 2510 Auto. Conduc. |
| This analysis is carried out electrode. | using proced | ures adapted from APHA Method 2510 "Conductivi | ity". Conductivity is determined using a conductivity |
| ECOLI-COLI-HLTH-VA | Water | E.coli by Colilert | APHA METHOD 9223 |
| determined simultaneously | r. The sample rs and then the | is mixed with a mixture hydrolyzable substrates an | ubstrate Coliform Test". E. coli and Total Coliform are d then sealed in a multi-well packet. The packet is e counted. The final result is obtained by comparing the |
| EPH-SF-FID-VA | Water | EPH in Water by GCFID | BCMOE EPH GCFID |
| Contaminated Sites "Extrace entire water sample with di | ctable Petrole chloromethan tion (GC/FID). | um Hydrocarbons in Water by GC/FID" (Version 2. e. The extract is then solvent exchanged to toluene EPH results include Polycyclic Aromatic Hydrocar | t, Lands and Parks (BCMELP) Analytical Method for 1, July 1999). The procedure involves extraction of the e and analysed by capillary column gas chromatography bons (PAH) and are therefore not equivalent to Light and |
| HARDNESS-CALC-VA | Water | Hardness | APHA 2340B |
| | | s) is calculated from the sum of Calcium and Magn entrations are preferentially used for the hardness | nesium concentrations, expressed in CaCO3 equivalents. calculation. |
| HG-DIS-CVAFS-VA | Water | Dissolved Mercury in Water by CVAFS | EPA SW-846 3005A & EPA 245.7 |
| American Public Health As States Environmental Prote involves a cold-oxidation of | sociation, and ection Agency f the acidified | with procedures adapted from "Test Methods for I (EPA). The procedures may involve preliminary s | nination of Water and Wastewater" published by the Evaluating Solid Waste" SW-846 published by the United ample treatment by filtration (EPA Method 3005A) and tion of the sample with stannous chloride. Instrumental |
| HG-TOT-CVAFS-VA | Water | Total Mercury in Water by CVAFS | EPA 245.7 |
| American Public Health As States Environmental Prote | sociation, and | with procedures adapted from "Test Methods for I (EPA). The procedure involves a cold-oxidation o | nination of Water and Wastewater" published by the Evaluating Solid Waste" SW-846 published by the United of the acidified sample using bromine monochloride prior to mic fluorescence spectrophotometry (EPA Method 245.7). |
| MET-DIS-ICP-VA | Water | Dissolved Metals in Water by ICPOES | EPA SW-846 3005A/6010B |
| This analysis is carried out | using proced | ures adapted from "Standard Methods for the Exan | nination of Water and Wastewater" published by the |

This analysis is carried out using procedures adapted from "Standard Methods for the Examination of Water and Wastewater" published by the American Public Health Association, and with procedures adapted from "Test Methods for Evaluating Solid Waste" SW-846 published by the United States Environmental Protection Agency (EPA). The procedure involves filtration (EPA Method 3005A) and analysis by inductively coupled plasma optical emission spectrophotometry (EPA Method 6010B).

Dissolved Metals in Water by ICPMS(Low) EPA SW-846 3005A/6020A This analysis is carried out using procedures adapted from "Standard Methods for the Examination of Water and Wastewater" published by the American Public Health Association, and with procedures adapted from "Test Methods for Evaluating Solid Waste" SW-846 published by the United States Environmental Protection Agency (EPA). The procedures involves preliminary sample treatment by filtration (EPA Method 3005A). Instrumental analysis is by inductively coupled plasma - mass spectrometry (EPA Method 6020A). Total Metals in Water by ICPOES EPA SW-846 3005A/6010B This analysis is carried out using procedures adapted from "Standard Methods for the Examination of Water and Wastewater" published by the

American Public Health Association, and with procedures adapted from "Test Methods for Evaluating Solid Waste" SW-846 published by the United States Environmental Protection Agency (EPA). The procedures may involve preliminary sample treatment by acid digestion, using either hotblock or microwave oven (EPA Method 3005A). Instrumental analysis is by inductively coupled plasma - optical emission spectrophotometry (EPA Method 6010B).

MET-TOT-LOW-MS-VA Water Total Metals in Water by ICPMS(Low) EPA SW-846 3005A/6020A This analysis is carried out using procedures adapted from "Standard Methods for the Examination of Water and Wastewater" published by the American Public Health Association, and with procedures adapted from "Test Methods for Evaluating Solid Waste" SW-846 published by the United States Environmental Protection Agency (EPA). The procedures may involve preliminary sample treatment by acid digestion, using either hotblock or microwave oven, or filtration (EPA Method 3005A). Instrumental analysis is by inductively coupled plasma - mass spectrometry (EPA Method 6020A).

APHA 4500-H "pH Value" PH-PCT-VA Water pH by Meter (Automated) This analysis is carried out using procedures adapted from APHA Method 4500-H "pH Value". The pH is determined in the laboratory using a pH electrode

It is recommended that this analysis be conducted in the field.

Water

Water

PH-PCT-VA Water

MET-DIS-LOW-MS-VA

MET-TOT-ICP-VA

TDS-VA

UV-ABS-VA

This analysis is carried out using procedures adapted from APHA Method 4500-H "pH Value". The pH is determined in the laboratory using a pH electrode

pH by Meter (Automated)

It is recommended that this analysis be conducted in the field.

S2-T-COL-VA Water Total Sulphide by Colorimetric This analysis is carried out using procedures adapted from APHA Method 4500-S2 "Sulphide". Sulphide is determined using the methlyene blue colourimetric method.

TCOLI-COLI-HLTH-VA Water Total coliform by Colilert

Water

This analysis is carried out using procedures adapted from APHA Method 9223 "Enzyme Substrate Coliform Test". E. coli and Total Coliform are determined simultaneously. The sample is mixed with a mixture hydrolyzable substrates and then sealed in a multi-well packet. The packet is incubated for 18 or 24 hours and then the number of wells exhibiting a positive response are counted. The final result is quantified by a statistical estimation of bacteria density (most probable number).

Total Dissolved Solids by Gravimetric

This analysis is carried out using procedures adapted from APHA Method 2540 "Solids". Solids are determined gravimetrically. Total Dissolved Solids (TDS) are determined by filtering a sample through a glass fibre filter, TDS is determined by evaporating the filtrate to dryness at 180 degrees celsius.

TURBIDITY-VA Water Turbidity by Meter

This analysis is carried out using procedures adapted from APHA Method 2130 "Turbidity". Turbidity is determined by the nephelometric method.

TURBIDITY-VA Water Turbidity by Meter

Water

This analysis is carried out using procedures adapted from APHA Method 2130 "Turbidity". Turbidity is determined by the nephelometric method.

This analysis is carried out using procedures adapted from APHA Method 5910B "Ultraviolet Absorption Method" and Method 415.3 "Determination of Total Organic Carbon and Specific UV Absorbance at 254nm in Source Water and Drinking Water", published by the United States Environmental Protection Agency (EPA). The sample is filtered through a 0.45um filter and measured for absorbance in a guartz cell at 254nm and reported as absorbance per cm (i.e. cm-1). The analysis is carried out without pH adjustment. Alternatively, results can be reported as % Transmittance (over one cm) where the absorbance result is converted to % Transmittance by the following calculation: %T = 100(10 to the power of -A). VOC7-HSMS-VA BTEX/MTBE/Styrene by Headspace GCMS EPA8260B, 5021 Water

The water sample, with added reagents, is heated in a sealed vial to equilibrium. The headspace from the vial is transfered into a gas chromatograph. Target compound concentrations are measured using mass spectrometry detection.

| VOC7/VOC-SURR-MS-VA | Water | VOC7 and/or VOC Surrogates for Waters | EPA8260B, 5021 |
|---------------------|-------|---------------------------------------|----------------|
| XYLENES-CALC-VA | Water | Sum of Xylene Isomer Concentrations | CALCULATION |

UV Absorbance by Spectrometry

Calculation of Total Xylenes

Total Xylenes is the sum of the concentrations of the ortho, meta, and para Xylene isomers. Results below detection limit (DL) are treated as zero. The DL for Total Xylenes is set to a value no less than the square root of the sum of the squares of the DLs of the individual Xylenes.

** ALS test methods may incorporate modifications from specified reference methods to improve performance.

Reference Information

L1096762 CONTD PAGE 5 of 6 28-DEC-11 15:26 (MT) Version[.] FINAI

APHA 2130 "Turbidity"

APHA 2130 Turbidity

APHA 4500-S2 Sulphide

APHA METHOD 9223

APHA 4500-H pH Value

APHA 2540 C - GRAVIMETRIC

APHA 5910B UV ABSORPTION METHOD

Reference Information

The last two letters of the above test code(s) indicate the laboratory that performed analytical analysis for that test. Refer to the list below:

Laboratory Definition Code Laboratory Location VA ALS ENVIRONMENTAL - VANCOUVER, BC, CANADA

Chain of Custody Numbers:

10-196045

GLOSSARY OF REPORT TERMS

Surrogate - A compound that is similar in behaviour to target analyte(s), but that does not occur naturally in environmental samples. For applicable tests, surrogates are added to samples prior to analysis as a check on recovery.

mg/kg - milligrams per kilogram based on dry weight of sample.

mg/kg wwt - milligrams per kilogram based on wet weight of sample.

mg/kg lwt - milligrams per kilogram based on lipid-adjusted weight of sample.

mg/L - milligrams per litre.

< - Less than.

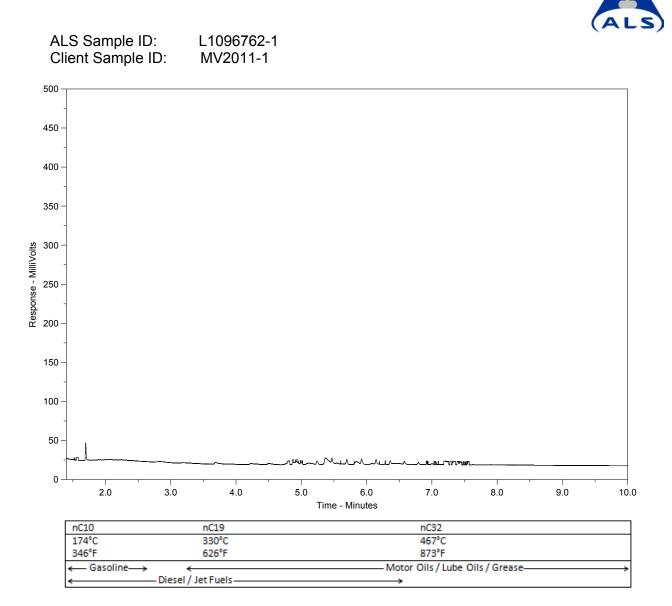
D.L. - The reported Detection Limit, also known as the Limit of Reporting (LOR).

N/A - Result not available. Refer to qualifier code and definition for explanation.

Test results reported relate only to the samples as received by the laboratory.

UNLESS OTHERWISE STATED, ALL SAMPLES WERE RECEIVED IN ACCEPTABLE CONDITION. Analytical results in unsigned test reports with the DRAFT watermark are subject to change, pending final QC review.

Hydrocarbon Distribution Report



The EPH Hydrocarbon Distribution Report (HDR) is intended to assist you in characterizing hydrocarbon products that may be present in your sample. For further interpretation, a current library of reference products is available on www.alsglobal.com or upon request.

The scale at the bottom of the chromatogram indicates the approximate retention times of common petroleum products, and three n-alkane hydrocarbon marker compounds. Retention times may vary between samples by as much as 0.5 minutes.

Peak heights in this report are a function of the sample concentration, the sample amount extracted, the sample dilution factor, and the response scale at the left.

A "-L-" in the sample ID denotes a low level sample. A "-S-" denotes a silica gel cleaned sample.

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PITEAU ASSOC. ENGINEERING LTD. ATTN: Kathy Tixier # 215 - 260 West Esplanade North Vancouver BC V7M 3G7 Date Received: 06-JAN-12 Report Date: 09-JAN-12 13:26 (MT) Version: FINAL

Client Phone: 604-986-8551

Certificate of Analysis

Lab Work Order #: L1102004

Project P.O. #: Job Reference: C of C Numbers: Legal Site Desc: NOT SUBMITTED 03131 10-196263

Mark

Brent Mack Account Manager

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L1102004 CONTD.... PAGE 2 of 3 09-JAN-12 13:26 (MT) Version: FINAL

| | Sample ID Description Sampled Date Sampled Time Client ID | L1102004-1 WATER 01-JAN-12 15:00 TW-01-SL-01 | | |
|------------------|---|--|--|--|
| Grouping | Analyte | | | |
| WATER | | | | |
| Physical Tests | Total Suspended Solids (mg/L) | 34.9 | | |
| Dissolved Metals | Iron (Fe)-Dissolved (mg/L) Manganese (Mn)-Dissolved (mg/L) | <0.030 | | |
| | Manganese (Min)-Dissolveu (mg/L) | 0.0361 | | |
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Reference Information

Test Method References:

| ALS Test Code | Matrix | Test Description | Method Reference** |
|---|---|--|---|
| MET-DIS-ICP-VA | Water | Dissolved Metals in Water by ICPOES | EPA SW-846 3005A/6010B |
| American Public Health | n Association, ar Protection Agenc | nd with procedures adapted from "Test Methods for cy (EPA). The procedure involves filtration (EPA M | amination of Water and Wastewater" published by the or Evaluating Solid Waste" SW-846 published by the United <i>I</i> lethod 3005A) and analysis by inductively coupled plasma |
| TSS-VA | Water | Total Suspended Solids by Gravimetric | APHA 2540 D - GRAVIMETRIC |
| This apply ais is corriad | out using proco | dure a dente d frage ADUA Mathead OF 40 "Calida" | O all de la service de la service a la service a la de la Tactal Ourse a service d |
| 5 | 01 | g a sample through a glass fibre filter, TSS is dete | Solids are determined gravimetrically. Total Suspended mined by drying the filter at 104 degrees celsius. |
| Solids (TSS) are deterr | nined by filtering | • | mined by drying the filter at 104 degrees celsius. |
| Solids (TŚS) are deterr * ALS test methods may | nined by filtering | a sample through a glass fibre filter, TSS is deten ifications from specified reference methods to imp | mined by drying the filter at 104 degrees celsius. |
| Solids (TŚS) are deterr * ALS test methods may | nined by filtering incorporate mod e above test coo | a sample through a glass fibre filter, TSS is deten ifications from specified reference methods to imp | mined by drying the filter at 104 degrees celsius. |
| Solids (TŚS) are deterr * ALS test methods may The last two letters of th | nined by filtering incorporate mod e above test cod Code Labor | a sample through a glass fibre filter, TSS is deter lifications from specified reference methods to imp de(s) indicate the laboratory that performed analy | rmined by drying the filter at 104 degrees celsius. prove performance. rtical analysis for that test. Refer to the list below: |

10-196263

GLOSSARY OF REPORT TERMS

Surrogate - A compound that is similar in behaviour to target analyte(s), but that does not occur naturally in environmental samples. For applicable tests, surrogates are added to samples prior to analysis as a check on recovery. mg/kg - milligrams per kilogram based on dry weight of sample.

mg/kg wwt - milligrams per kilogram based on wet weight of sample.

mg/kg lwt - milligrams per kilogram based on lipid-adjusted weight of sample.

mg/L - milligrams per litre.

< - Less than.

D.L. - The reported Detection Limit, also known as the Limit of Reporting (LOR).

N/A - Result not available. Refer to qualifier code and definition for explanation.

Test results reported relate only to the samples as received by the laboratory.

UNLESS OTHERWISE STATED, ALL SAMPLES WERE RECEIVED IN ACCEPTABLE CONDITION.

Analytical results in unsigned test reports with the DRAFT watermark are subject to change, pending final QC review.

10-196263

Chain of Custody / Analytical Request Form Canada Toll Free: 1 800 668 9878

ALS Environmental

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PITEAU ASSOC. ENGINEERING LTD. ATTN: Kathy Tixier # 215 - 260 West Esplanade North Vancouver BC V7M 3G7 Date Received:18-JAN-12Report Date:25-JAN-12 13:24 (MT)Version:FINAL

Client Phone: 604-986-8551

Certificate of Analysis

Lab Work Order #: L1105481

Project P.O. #: Job Reference: C of C Numbers: Legal Site Desc: NOT SUBMITTED 3131 10-196227

Mark

Brent Mack Account Manager

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L1105481 CONTD.... PAGE 2 of 7 25-JAN-12 13:24 (MT) Version: FINAL

| | Sample ID Description Sampled Date Sampled Time Client ID | L1105481-1 WATER 18-JAN-12 14:30 TW12-1 | | |
|--------------------------|---|---|--|--|
| Grouping | Analyte | | | |
| WATER | | | | |
| Physical Tests | Colour, True (CU) | <5.0 | | |
| | Conductivity (uS/cm) | 64.5 | | |
| | Hardness (as CaCO3) (mg/L) | 46.3 | | |
| | рН (рН) | 7.60 | | |
| | Total Dissolved Solids (mg/L) | 41 | | |
| | Turbidity (NTU) | 0.23 | | |
| Anions and Nutrients | Alkalinity, Total (as CaCO3) (mg/L) | 47.3 | | |
| | Chloride (Cl) (mg/L) | 0.70 | | |
| | Fluoride (F) (mg/L) | 0.030 | | |
| | Nitrate (as N) (mg/L) | 0.0729 | | |
| | Nitrite (as N) (mg/L) | <0.0010 | | |
| | Sulfate (SO4) (mg/L) | 10.7 | | |
| | Sulphide as S (mg/L) | <0.0020 | | |
| Bacteriological Tests | E. coli (MPN/100mL) Coliform Bacteria - Total (MPN/100mL) | <1 | | |
| Total Metals | Aluminum (Al)-Total (mg/L) | <1 | | |
| | Antimony (Sb)-Total (mg/L) | <0.010 | | |
| | Arsenic (As)-Total (mg/L) | <0.00050 0.00167 | | |
| | Barium (Ba)-Total (mg/L) | <0.020 | | |
| | Boron (B)-Total (mg/L) | <0.020 | | |
| | Cadmium (Cd)-Total (mg/L) | <0.0020 | | |
| | Calcium (Ca)-Total (mg/L) | 14.6 | | |
| | Chromium (Cr)-Total (mg/L) | <0.0020 | | |
| | Copper (Cu)-Total (mg/L) | 0.0033 | | |
| | Iron (Fe)-Total (mg/L) | <0.030 | | |
| | Lead (Pb)-Total (mg/L) | 0.0120 | | |
| | Magnesium (Mg)-Total (mg/L) | 2.45 | | |
| | Manganese (Mn)-Total (mg/L) | 0.0798 | | |
| | Mercury (Hg)-Total (mg/L) | <0.00020 | | |
| | Potassium (K)-Total (mg/L) | 0.87 | | |
| | Selenium (Se)-Total (mg/L) | <0.0010 | | |
| | Sodium (Na)-Total (mg/L) | <2.0 | | |
| | Uranium (U)-Total (mg/L) | <0.00010 | | |
| | Zinc (Zn)-Total (mg/L) | <0.050 | | |
| Dissolved Metals | Aluminum (Al)-Dissolved (mg/L) | <0.010 | | |
| | Antimony (Sb)-Dissolved (mg/L) | <0.00050 | | |

L1105481 CONTD.... PAGE 3 of 7 25-JAN-12 13:24 (MT) Version: FINAL

| | Sample ID Description Sampled Date Sampled Time Client ID | L1105481-1 WATER 18-JAN-12 14:30 TW12-1 | | |
|--|---|---|--|--|
| Grouping | Analyte | | | |
| WATER | | | | |
| Dissolved Metals | Arsenic (As)-Dissolved (mg/L) | 0.00181 | | |
| | Barium (Ba)-Dissolved (mg/L) | <0.020 | | |
| | Boron (B)-Dissolved (mg/L) | <0.10 | | |
| | Cadmium (Cd)-Dissolved (mg/L) | <0.00020 | | |
| | Calcium (Ca)-Dissolved (mg/L) | 14.6 | | |
| | Chromium (Cr)-Dissolved (mg/L) | <0.0020 | | |
| | Copper (Cu)-Dissolved (mg/L) | <0.0010 | | |
| | Iron (Fe)-Dissolved (mg/L) | <0.030 | | |
| | Lead (Pb)-Dissolved (mg/L) | 0.00114 | | |
| | Magnesium (Mg)-Dissolved (mg/L) | 2.38 | | |
| | Manganese (Mn)-Dissolved (mg/L) | 0.0764 | | |
| | Mercury (Hg)-Dissolved (mg/L) | <0.00020 | | |
| | Potassium (K)-Dissolved (mg/L) | 0.86 | | |
| | Selenium (Se)-Dissolved (mg/L) | <0.0010 | | |
| | Sodium (Na)-Dissolved (mg/L) | <2.0 | | |
| | Uranium (U)-Dissolved (mg/L) | <0.00010 | | |
| | Zinc (Zn)-Dissolved (mg/L) | <0.050 | | |
| Volatile Organic Compounds | Benzene (mg/L) | <0.00050 | | |
| | Ethylbenzene (mg/L) | <0.00050 | | |
| | Methyl t-butyl ether (MTBE) (mg/L) | <0.00050 | | |
| | Styrene (mg/L) | <0.00050 | | |
| | Toluene (mg/L) | <0.00050 | | |
| | ortho-Xylene (mg/L) | <0.00050 | | |
| | meta- & para-Xylene (mg/L) | <0.00050 | | |
| | Xylenes (mg/L) | <0.00075 | | |
| | Surrogate: 4-Bromofluorobenzene (SS) (%) | 93.9 | | |
| | Surrogate: 1,4-Difluorobenzene (SS) (%) | 99.0 | | |
| Hydrocarbons | EPH10-19 (mg/L) | <0.25 | | |
| | EPH19-32 (mg/L) | <0.25 | | |
| | LEPH (mg/L) | <0.25 | | |
| | HEPH (mg/L) | <0.25 | | |
| | Volatile Hydrocarbons (VH6-10) (mg/L) | <0.10 | | |
| | VPH (C6-C10) (mg/L) | <0.10 | | |
| | Surrogate: 3,4-Dichlorotoluene (SS) (%) | 102.2 | | |
| Polycyclic Aromatic Hydrocarbons | Acenaphthene (mg/L) | <0.000050 | | |
| | Acenaphthylene (mg/L) | <0.000050 | | |

L1105481 CONTD.... PAGE 4 of 7 25-JAN-12 13:24 (MT) Version: FINAL

| | Sample ID Description Sampled Date Sampled Time Client ID | L1105481-1 WATER 18-JAN-12 14:30 TW12-1 | | |
|--|---|---|--|--|
| Grouping | Analyte | | | |
| WATER | | | | |
| Polycyclic Aromatic Hydrocarbons | Acridine (mg/L) | <0.000050 | | |
| | Anthracene (mg/L) | <0.000050 | | |
| | Benz(a)anthracene (mg/L) | <0.000050 | | |
| | Benzo(a)pyrene (mg/L) | <0.000010 | | |
| | Benzo(b)fluoranthene (mg/L) | <0.000050 | | |
| | Benzo(g,h,i)perylene (mg/L) | <0.000050 | | |
| | Benzo(k)fluoranthene (mg/L) | <0.000050 | | |
| | Chrysene (mg/L) | <0.000050 | | |
| | Dibenz(a,h)anthracene (mg/L) | <0.000050 | | |
| | Fluoranthene (mg/L) | <0.000050 | | |
| | Fluorene (mg/L) | <0.000050 | | |
| | Indeno(1,2,3-c,d)pyrene (mg/L) | <0.000050 | | |
| | Naphthalene (mg/L) | <0.000050 | | |
| | Phenanthrene (mg/L) | <0.000050 | | |
| | Pyrene (mg/L) | <0.000050 | | |
| | Quinoline (mg/L) | <0.000050 | | |
| | Surrogate: Acenaphthene d10 (%) | 93.4 | | |
| | Surrogate: Acridine d9 (%) | 103.6 | | |
| | Surrogate: Chrysene d12 (%) | 94.9 | | |
| | Surrogate: Naphthalene d8 (%) | 93.8 | | |
| | Surrogate: Phenanthrene d10 (%) | 71.8 | | |
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Reference Information

Test Method References: ALS Test Code Matrix Method Reference** **Test Description** ALK-COL-VA Water Alkalinity by Colourimetric (Automated) APHA 310.2 This analysis is carried out using procedures adapted from EPA Method 310.2 "Alkalinity". Total Alkalinity is determined using the methyl orange colourimetric method. ANIONS-CL-IC-VA Water Chloride by Ion Chromatography APHA 4110 B. This analysis is carried out using procedures adapted from APHA Method 4110 B. "Ion Chromatography with Chemical Suppression of Eluent Conductivity" and EPA Method 300.0 "Determination of Inorganic Anions by Ion Chromatography" ANIONS-F-IC-VA Water Fluoride by Ion Chromatography APHA 4110 B. This analysis is carried out using procedures adapted from APHA Method 4110 B. "Ion Chromatography with Chemical Suppression of Eluent Conductivity" and EPA Method 300.0 "Determination of Inorganic Anions by Ion Chromatography". Nitrite in Water by Ion Chromatography ANIONS-NO2-IC-VA Water FPA 300 0 This analysis is carried out using procedures adapted from EPA Method 300.0 "Determination of Inorganic Anions by Ion Chromatography". Nitrite is detected by UV absorbance. ANIONS-NO3-IC-VA Water Nitrate in Water by Ion Chromatography FPA 300.0 This analysis is carried out using procedures adapted from EPA Method 300.0 "Determination of Inorganic Anions by Ion Chromatography". Nitrate is detected by UV absorbance. ANIONS-SO4-IC-VA APHA 4110 B. Water Sulfate by Ion Chromatography This analysis is carried out using procedures adapted from APHA Method 4110 B. "Ion Chromatography with Chemical Suppression of Eluent Conductivity" and EPA Method 300.0 "Determination of Inorganic Anions by Ion Chromatography". COLOUR-TRUE-VA Water Colour (True) by Spectrometer BCMOE Colour Single Wavelength This analysis is carried out using procedures adapted from British Columbia Environmental Manual "Colour- Single Wavelength." Colour (True Colour) is determined by filtering a sample through a 0.45 micron membrane filter followed by analysis of the filtrate using the platinum-cobalt colourimetric method. Aparent Colour is determined without prior sample filtration. Colour is pH dependent. Unless otherwise indicated, reported colour results pertain to the pH of the sample as received, to within +/- 1 pH unit. EC-PCT-VA Water Conductivity (Automated) APHA 2510 Auto, Conduc, This analysis is carried out using procedures adapted from APHA Method 2510 "Conductivity". Conductivity is determined using a conductivity electrode. **ECOLI-COLI-HLTH-VA** Water E.coli by Colilert APHA METHOD 9223 This analysis is carried out using procedures adapted from APHA Method 9223 "Enzyme Substrate Coliform Test". E. coli and Total Coliform are determined simultaneously. The sample is mixed with a mixture hydrolyzable substrates and then sealed in a multi-well packet. The packet is incubated for 18 or 24 hours and then the number of wells exhibiting a positive response are counted. The final result is obtained by comparing the positive responses to a probability table. EPH-SF-FID-VA Water EPH in Water by GCFID BCMOE EPH GCFID This analysis is carried out in accordance with the British Columbia Ministry of Environment, Lands and Parks (BCMELP) Analytical Method for Contaminated Sites "Extractable Petroleum Hydrocarbons in Water by GC/FID" (Version 2.1, July 1999). The procedure involves extraction of the entire water sample with dichloromethane. The extract is then solvent exchanged to toluene and analysed by capillary column gas chromatography with flame ionization detection (GC/FID). EPH results include Polycyclic Aromatic Hydrocarbons (PAH) and are therefore not equivalent to Light and Heavy Extractable Petroleum Hydrocarbons (LEPH/HEPH). HARDNESS-CALC-VA Water Hardness APHA 2340B Hardness (also known as Total Hardness) is calculated from the sum of Calcium and Magnesium concentrations, expressed in CaCO3 equivalents. Dissolved Calcium and Magnesium concentrations are preferentially used for the hardness calculation. Dissolved Mercury in Water by CVAFS **HG-DIS-CVAFS-VA** Water FPA SW-846 3005A & FPA 245.7 This analysis is carried out using procedures adapted from "Standard Methods for the Examination of Water and Wastewater" published by the American Public Health Association, and with procedures adapted from "Test Methods for Evaluating Solid Waste" SW-846 published by the United States Environmental Protection Agency (EPA). The procedures may involve preliminary sample treatment by filtration (EPA Method 3005A) and involves a cold-oxidation of the acidified sample using bromine monochloride prior to reduction of the sample with stannous chloride. Instrumental analysis is by cold vapour atomic fluorescence spectrophotometry (EPA Method 245.7). **HG-TOT-CVAFS-VA** EPA 245.7 Water Total Mercury in Water by CVAFS This analysis is carried out using procedures adapted from "Standard Methods for the Examination of Water and Wastewater" published by the American Public Health Association, and with procedures adapted from "Test Methods for Evaluating Solid Waste" SW-846 published by the United States Environmental Protection Agency (EPA). The procedure involves a cold-oxidation of the acidified sample using bromine monochloride prior to reduction of the sample with stannous chloride. Instrumental analysis is by cold vapour atomic fluorescence spectrophotometry (EPA Method 245.7). LEPH/HEPH-CALC-VA Water LEPHs and HEPHs BC MOE LABORATORY MANUAL (2005) Light and Heavy Extractable Petroleum Hydrocarbons in water. These results are determined according to the British Columbia Ministry of Environment, Lands, and Parks Analytical Method for Contaminated Sites "Calculation of Light and Heavy Extractable Petroleum Hydrocarbons in Solids or Water". According to this method, LEPH and HEPH are calculated by subtracting selected Polycyclic Aromatic Hydrocarbon results from Extractable Petroleum Hydrocarbon results. To calculate LEPH, the individual results for Acenaphthene, Acridine, Anthracene, Fluorene, Naphthalene

Extractable Petroleum Hydrocarbon results. I o calculate LEPH, the individual results for Acenaphthene, Acridine, Anthracene, Fluorene, Naphthalen and Phenanthrene are subtracted from EPH(C10-19). To calculate HEPH, the individual results for Benz(a)anthracene, Benzo(a)pyrene, Fluoranthene, and Pyrene are subtracted from EPH(C19-32). Analysis of Extractable Petroleum Hydrocarbons adheres to all prescribed elements of the BCMELP method "Extractable Petroleum Hydrocarbons in Water by GC/FID" (Version 2.1, July 20, 1999).

| | | Reference Informatio | n | 25-JAN-12 13:2 | 24 (MT) |
|---|---------------------------------|---|--|--|------------------|
| | | | | Version: | FINAL |
| | | | | | |
| MET-DIS-ICP-VA | Water | Dissolved Metals in Water by ICPOES | EPA SW-846 3005A/6010B | | |
| American Public Health As | sociation, and ection Agency | ures adapted from "Standard Methods for the Examination d with procedures adapted from "Test Methods for Evaluat (EPA). The procedure involves filtration (EPA Method 3 A Method 6010B). | ating Solid Waste" SW-846 p | ublished by the U | nited |
| MET-DIS-LOW-MS-VA | Water | Dissolved Metals in Water by ICPMS(Low) | EPA SW-846 3005A/6020A | | |
| American Public Health As States Environmental Prote | sociation, and ection Agency | ures adapted from "Standard Methods for the Examination d with procedures adapted from "Test Methods for Evaluat (EPA). The procedures involves preliminary sample tre upled plasma - mass spectrometry (EPA Method 6020A | ating Solid Waste" SW-846 pe atment by filtration (EPA Meth | ublished by the U | nited |
| MET-TOT-ICP-VA | Water | Total Metals in Water by ICPOES | EPA SW-846 3005A/6010B | | |
| American Public Health As States Environmental Prote | sociation, and ection Agency | ures adapted from "Standard Methods for the Examination with procedures adapted from "Test Methods for Evaluate (EPA). The procedures may involve preliminary sample Instrumental analysis is by inductively coupled plasma - | ating Solid Waste" SW-846 per e treatment by acid digestion, | ublished by the U using either hotb | nited lock or |
| MET-TOT-LOW-MS-VA | Water | Total Metals in Water by ICPMS(Low) | EPA SW-846 3005A/6020A | | |
| American Public Health As States Environmental Prote | sociation, and ection Agency | ures adapted from "Standard Methods for the Examination d with procedures adapted from "Test Methods for Evaluate (EPA). The procedures may involve preliminary sample ad 3005A). Instrumental analysis is by inductively couple | ating Solid Waste" SW-846 protection streatment by acid digestion, | ublished by the U using either hotb | ock or |
| PAH-SF-MS-VA | Water | PAH in Water by GCMS | EPA 3510, 8270 | | |
| | | n dichloromethane, prior to analysis by gas chromatogra dily chromatographically separated, benzo(j)fluoranthene | | | |
| PAH-SURR-MS-VA | Water | PAH Surrogates for Waters | EPA 3510, 8270 | | |
| Analysed as per the corres demonstrate analytical acc | | test method. Known quantities of surrogate compounds | are added prior to analysis to | each sample to | |
| PH-PCT-VA | Water | pH by Meter (Automated) | APHA 4500-H "pH Value" | | |
| This analysis is carried out electrode | using proced | ures adapted from APHA Method 4500-H "pH Value". Th | ne pH is determined in the lab | oratory using a p | Н |
| It is recommended that this | analysis be d | conducted in the field. | | | |
| PH-PCT-VA | Water | pH by Meter (Automated) | APHA 4500-H pH Value | | |
| This analysis is carried out electrode | using proced | ures adapted from APHA Method 4500-H "pH Value". Th | ne pH is determined in the lab | oratory using a p | Н |
| It is recommended that this | analysis be o | conducted in the field. | | | |
| S2-L-T-COL-VA | Water | Total Sulphide Low Level by Colorimetric | APHA 4500-S2 Sulphide | | |
| This analysis is carried out colourimetric method. | using proced | ures adapted from APHA Method 4500-S2 "Sulphide". S | ulphide is determined using the | ne methlyene blu | Э |
| TCOLI-COLI-HLTH-VA | Water | Total coliform by Colilert | APHA METHOD 9223 | | |
| determined simultaneously | . The sample s and then th | ures adapted from APHA Method 9223 "Enzyme Substra is mixed with a mixture hydrolyzable substrates and ther e number of wells exhibiting a positive response are cou- bable number). | n sealed in a multi-well packet | t. The packet is | |
| TDS-VA | Water | Total Dissolved Solids by Gravimetric | APHA 2540 C - GRAVIMET | RIC | |
| | | ures adapted from APHA Method 2540 "Solids". Solids a ole through a glass fibre filter, TDS is determined by evap | | | |
| TURBIDITY-VA | Water | Turbidity by Meter | APHA 2130 "Turbidity" | | |
| This analysis is carried out | using proced | ures adapted from APHA Method 2130 "Turbidity". Turbi | dity is determined by the nept | nelometric metho | d. |
| TURBIDITY-VA | Water | Turbidity by Meter | APHA 2130 Turbidity | | |
| | using proced | ures adapted from APHA Method 2130 "Turbidity". Turbi | | nelometric metho | d. |
| VH-HSFID-VA | Water | VH in Water by Headspace GCFID | B.C. MIN. OF ENV. LAB. M | AN. (2009) | |
| | | is heated in a sealed vial to equilibrium. The headspace nd n-decane are measured and summed together using | | o a gas chromato | graph. |
| VH-SURR-FID-VA | Water | VH Surrogates for Waters | B.C. MIN. OF ENV. LAB. M | AN. (2009) | |

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Reference Information

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| VOC7-HSMS-VA | Water | BTEX/MTBE/Styrene by Headspace GCMS | EPA8260B, 5021 |
|--------------------------------|--------------------------------|---|---|
| | | , is heated in a sealed vial to equilibrium. The head easured using mass spectrometry detection. | Ispace from the vial is transfered into a gas chromatograph |
| VOC7/VOC-SURR-MS-VA | Water | VOC7 and/or VOC Surrogates for Waters | EPA8260B, 5021 |
| VPH-CALC-VA | Water | VPH is VH minus select aromatics | BC MOE LABORATORY MANUAL (2005) |
| Volatile Petroleum Hydroca | rbons in Sol , in solids, S | ids or Water". The concentrations of specific Mono | nalytical Method for Contaminated Sites "Calculation of cyclic Aromatic Hydrocarbons (Benzene, Toluene, ation of Volatile Hydrocarbons (VH) that elute between n- |
| XYLENES-CALC-VA | Water | Sum of Xylene Isomer Concentrations | CALCULATION |
| Calculation of Total Xylenes | 6 | | |
| | | rations of the ortho, meta, and para Xylene isomers ue no less than the square root of the sum of the sc | B. Results below detection limit (DL) are treated as zero. luares of the DLs of the individual Xylenes. |
| ** ALS test methods may incom | rporate mod | ifications from specified reference methods to impr | ove performance. |
| The last two letters of the ab | ove test co | de(s) indicate the laboratory that performed analyti | cal analysis for that test. Refer to the list below: |
| Laboratory Definition Code | e Labor | atory Location | |
| VA | ALS E | NVIRONMENTAL - VANCOUVER, BC, CANADA | |
| | | | |

10-196227

GLOSSARY OF REPORT TERMS

Surrogate - A compound that is similar in behaviour to target analyte(s), but that does not occur naturally in environmental samples. For applicable tests, surrogates are added to samples prior to analysis as a check on recovery.

mg/kg - milligrams per kilogram based on dry weight of sample.

mg/kg wwt - milligrams per kilogram based on wet weight of sample.

mg/kg lwt - milligrams per kilogram based on lipid-adjusted weight of sample.

mg/L - milligrams per litre.

< - Less than.

D.L. - The reported Detection Limit, also known as the Limit of Reporting (LOR).

N/A - Result not available. Refer to qualifier code and definition for explanation.

Test results reported relate only to the samples as received by the laboratory.

UNLESS OTHERWISE STATED, ALL SAMPLES WERE RECEIVED IN ACCEPTABLE CONDITION.

Analytical results in unsigned test reports with the DRAFT watermark are subject to change, pending final QC review.

SRC ANALYTICAL

Jan 25, 2012

422 Downey Road Saskatoon, Saskatchewan, Canada S7N 4N1 (306) 933-6932 or 1-800-240-8808

ALS Laboratory Group 8081 Lougheed Hwy Burnaby, BC V5A 1W9 Attn: Brent Mack

Date Samples Received: Jan-20-2012

Client P.O.: L1105481

This is a final report.

Organics results have been authorized by Pat Moser, Supervisor

ICP results have been authorized by Keith Gipman, Supervisor

Inorganics and Radiochemistry results have been authorized by Jeff Zimmer, Supervisor

SLOWPOKE-2 results have been authorized by Dave Chorney

* Test methods and data are validated by the laboratory's Quality Assurance Program.

* Routine methods follow recognized procedures from sources such as

- * Standard Methods for the Examination of Water and Wastewater APHA AWWA WEF
- * Environment Canada
- * US EPA
- * CANMET

* The results reported relate only to the test samples as provided by the client.

* Samples will be kept for 30 days after the final report is sent. Please contact the lab if you have any special requirements.

* Additional information is available upon request.

SRC ANALYTICAL

Jan 25, 2012

422 Downey Road Saskatoon, Saskatchewan, Canada S7N 4N1 (306) 933-6932 or 1-800-240-8808

ALS Laboratory Group 8081 Lougheed Hwy Burnaby, BC V5A 1W9 Attn: Brent Mack

Date Samples Received: Jan-20-2012

Client P.O.: L1105481

2903 01/18/2012 L1105481-1 TW12-1 *WATER*

| Analyte | Units | 2903 |
|----------------------------|-------------------|--|
| Radio Chemistry | | |
| Gross alpha | Bq/L | <0.05 |
| Gross beta | Bq/L | 0.05 |
| Symbol of "<" means above. | "less than". This | indicates that it was not detected at level stated |



422 Downey Road Saskatoon, Saskatchewan, Canada S7N 4N1 (306) 933-6932 or 1-800-240-8808

Quality Control Report

Brent Mack ALS Laboratory Group 8081 Lougheed Hwy Burnaby, BC V5A 1W9

This report was generated for samples included in SRC Group # 2012-702

Reference Materials and Standards:

A reference material of known concentration is used whenever possible as either a control sample or control standard and analyzed with each batch of samples. These "QC" results are used to assess the performance of the method and must be within clearly defined limits; otherwise corrective action is required.

| QC Analysis | Units | Target Value | Obtained Value |
|-------------|-------|--------------|-----------------------|
| Gross Alpha | Bq/L | 14.6 | 13.9 |
| Gross Alpha | Bq | 1.9 | 1.5 |
| Gross Beta | Bq/L | 12.4 | 12.5 |
| Gross Beta | Bq | 1.98 | 1.91 |

Duplicates:

Duplicates are used to assess problems with precision and help ensure that samples within a given batch were processed appropriately. The difference between duplicates must be within strict limits, otherwise corrective action is required. Please note, the duplicate(s) in this report are duplicates analyzed within a given batch of test samples and may not be from this specific group of samples.

| Duplicate Analysis | Units | First Result | Second Result |
|--------------------|-------|---------------------|---------------|
| Gross alpha | Bq/L | < 0.12 | < 0.12 |
| Gross beta | Bq/L | 0.06 | 0.05 |

Spikes and/or Surrogates:

Samples spiked with a known quantity of the analyte of interest or a surrogate which is a known quantity of a compound which behaves in a similar manner to the analyte of interest, are used to assess problems with the sample processing or sample matrix. The recovery must be within clearly defined limits when the quantity of spike is comparable to the sample concentration.

Jan 25, 2012

SRC ANALYTICAL

This report was generated for samples included in SRC Group # 2012-702

Jan 25, 2012

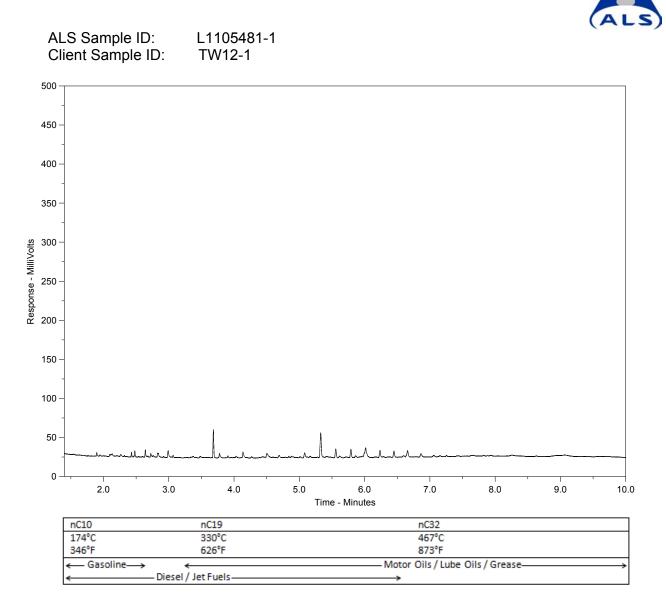
| Spike Analysis | Percent Recovery | | | | | |
|---------------------------|------------------|------|--|--|--|--|
| Gross Alpha Gross Beta | 153 90 | *(1) | | | | |

*(1) The percent recovery for Gross alpha in the spiked sample was just outside the laboratory's specified limits of 50 - 150% recovery. The data was reviewed and additional quality control measures in the same batch were within specified limits.

Overall, there were no other indications of problems with the analysis and the results were considered acceptable.

Roxane Ortmann - Quality Assurance Supervisor

Hydrocarbon Distribution Report



The EPH Hydrocarbon Distribution Report (HDR) is intended to assist you in characterizing hydrocarbon products that may be present in your sample. For further interpretation, a current library of reference products is available on www.alsglobal.com or upon request.

The scale at the bottom of the chromatogram indicates the approximate retention times of common petroleum products, and three n-alkane hydrocarbon marker compounds. Retention times may vary between samples by as much as 0.5 minutes.

Peak heights in this report are a function of the sample concentration, the sample amount extracted, the sample dilution factor, and the response scale at the left.

A "-L-" in the sample ID denotes a low level sample. A "-S-" denotes a silica gel cleaned sample.

10-196227

Chain of Custody / Analytical Request Form Canada Toll Free: 1 800 668 9878

| ALS | Environmen | tal | | | www.a | alsglobal.com | | | | | | | | | | Pa | ige _ | of | |
|--|---|----------------------------------|----------------------------------|----------------------------------|-------------------------------|-------------------|-----------------------|--------------|--|--------------|------------|------------|----------|------------|----------|-----------|---------------|---------------------------------------|-----------|
| Report To | KATHY TI | XIER | | Report Format / Distribution Ser | | | | | Service Request: (Rush subject to availability - Contact ALS to confirm TAT) | | | | | | | | | | |
| Company: | PITEAU A | SSO CIAT | ES | Standar | rd: Other (s | pecify): | | 7 | Regula | ar (Stand | nuT brai | naround | Times - | Busines | s Days) | | | | |
| Contact: | | <u> </u> | ····· | Select: | | Digital | Fax | | Priority(2-4 Business Days)-50% surcharge - Contact ALS to confirm TAT | | | | | | | | | | |
| Address: | 215-360 | WESP | lanade | | Email 1: Ktixier @ piteau.com | | | | | ency (1- | 2 Busine | ess Days | s)-100% | Surcha | rge - Co | ntact Al. | .S te confi | irm TAT | |
| | North V | | | Email 2 | r segovi | a e' pite | av-com | | Same | Day or V | Veekend | l Emerge | ency • C | ontact A | LS to co | nfirm T/ | AT | | |
| Phone: 60 | | Fax: | | | | | <u>.</u> | | | | | | Analy | sis Re | ques | t | | | |
| involce To | Same as Report ? (circle |) (Testor No (if | No, provide detai | ls) Client / | Project Information | | | | | (| Indica | ate Filt | ered | or Pres | served | i, F/P |) | | |
| | Copy of Invoice with Rep | oort? (circle) Yes | s or No | Job #: | 3131 | | · | \checkmark | P | 10 | 2 | 2 | ∕₽ | 1 | 2 | \leq | | \leq | 1 |
| Company: | | · | | PO / AF | =E: | | · | 4 | 1 | [•] | | | 28 | • | | | 1 | | |
| Contact: | | | <u> </u> | LSD: | | | | 4 | | | ~~ | . . | E S | \sim | ব | | | | |
| Address: | | | | | | | | 1- | 4 | 5 | -4 | 7 | 25 | <u>א</u> ' | 4 | 1 | | | ers. |
| Phone: | • | Fax: | | Quote # | ote #: | | | | | 1 X | Meta | 41 | 7 ¢ | R | 41 | 13 | | | tair |
| Lab Work O | rder # (lab use only) | LII | 05481 | ALS Contac | it: . | Sampler: | | e rera | I | Bacteria | 5 | 2/2 | 151 1 | c (B | s s | r Ho | | | er of Cor |
| Sample # | (This | Sample Ide description will a | ntification appear on the rep | ort) - | Date (dd-mmm-yy) | Time (hb:mm) | Sample Type | 9 | tot | Ņ | Zid | S | HEP. | ٥> > | 619 | 0 | | | Numbe |
| | | W12- | 1 | | 18 JAH 12 | 2:30 | Water | 1 | | 1 | l | 1 | l | | 1 | 3 | | | |
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| | | | Failure to c | complete all portio | ns of this form may | delay analysis. | Please fill in this f | orm L | EGIBI | LY. | | | | | | | | | |
| | By t | he use of this fo | orm the user ack | nowledges and ag | rees with the Terms | and Conditions | as specified on th | ie bacl | k pag | e of th | e wh | ite - re | eport o | сору. | | | | | |
| · · · | SHIPMENT RELEA | SE (client use) | | | SHIPMENT RECEPT | ION (lab use only | | T | | | SHIP | MENT | VERI | FICAT | ION (l | ab us | e only) | | |
| Released by | Seguia | Date: JAN19 | Time: 4:30 | Received by: | Date: 18Ten 12 | Time: 16.51 | Temperature: | | fied by | y: | | Date | 1. | | Time | | Y | Observatio Yes / No ? | ? |
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Report ID:EMSR0300Report Time:2012- 02- 14 03:31Requested by:

Selection Criteria

EMS ID: E217927 Region: Permit ID: Office: Establishing Agency: Location Type: Monitoring Group: Collection Start Date/Time From 1950-01-01 00:00 Collection Start Date/Time To: 2012-02-14 23:59 Specific Month: Sample State: Sample Descriptor: Study: Parameter Code: Data Index: QA Index: Watershed: Eco Region:

Order by

| Office Code: | Y |
|--------------------|---|
| Parameter Code: | Y |
| Location Type: | N |
| Sample State Code: | N |

Environmental Monitoring System Historical Statistics Report

Name: FVGMP STAVE LAKE ROAD, MISSION

Page 2 of 4

Monitoring Location: E217927

Office: LOWER MAINLAND

Establishing Agency: WMB-Groundwater

Watershed Code:

Latitude: 049:15:27 Longitude: 122:14:37

| Location Type: | WELL | |
|------------------|--------------|------------|
| No. Samples: | 2 | |
| First Collection | Date: | 1992-12-08 |
| Most Recent Coll | ection Date: | 1993-09-01 |

| Sa | mple | Param | eter | Units | Maximum | | Minimum | Mean | Median | Standard | Latest Date | Total | |
|-------|-------|-------|---------------------------|----------|----------|---|----------|----------|----------|-----------|-------------|-------|------|
| State | Desc. | | | | | | | | | Deviation | | Obs. | Obs. |
| FW | GE | 0002 | Color True | Col.unit | 5.00000 | < | 5.00000 | | 5.00000 | | 1993-09-01 | 2 | 0 |
| FW | GE | 0004 | рH | pH units | 6.70000 | | 6.70000 | 6.70000 | 6.70000 | 0.00000 | 1993-09-01 | 2 | 2 |
| FW | GE | 0007 | Residue: Filterable 1.0u | mg/L | 50.00000 | | 45.00000 | 47.50000 | 47.50000 | 3.53553 | 1993-09-01 | 2 | 2 |
| FW | GE | 0011 | Specific Conductance | uS/cm | 53.00000 | | 52.00000 | 52.50000 | 52.50000 | 0.70711 | 1993-09-01 | 2 | 2 |
| FW | GE | 0015 | Turbidity | NTU | 0.60000 | | 0.60000 | 0.60000 | 0.60000 | 0.00000 | 1992-12-08 | 1 | 1 |
| FW | GE | 0101 | Alkalinity Phen. 8.3 | mg/L | 0.50000 | < | 0.50000 | | 0.50000 | | 1993-09-01 | 2 | 0 |
| FW | GE | 0102 | Alkalinity Total 4.5 | mg/L | 22.30000 | | 20.60000 | 21.45000 | 21.45000 | 1.20208 | 1993-09-01 | 2 | 2 |
| FW | GE | 0107 | Hardness Total (Total) | mg/L | 20.20000 | | 19.20000 | 19.70000 | 19.70000 | 0.70711 | 1993-09-01 | 2 | 2 |
| FW | GE | 0112 | Nitrogen Organic-Total | mg/L | 0.04000 | | 0.04000 | 0.04000 | 0.04000 | 0.00000 | 1992-12-08 | 1 | 1 |
| FW | GE | 0150 | Bicarbonate Alkalinity | mg/L | 22.30000 | | 20.60000 | 21.45000 | 21.45000 | 1.20208 | 1993-09-01 | 2 | 2 |
| FW | GE | 1104 | Chloride Dissolved | mg/L | 0.90000 | | 0.70000 | 0.80000 | 0.80000 | 0.14142 | 1993-09-01 | 2 | 2 |
| FW | GE | 1106 | Fluoride Dissolved | mg/L | 0.10000 | < | 0.10000 | | 0.10000 | | 1993-09-01 | 2 | 0 |
| FW | GE | 1107 | Hardness Total (Dissolved | mg/L | 20.00000 | | 19.80000 | 19.90000 | 19.90000 | 0.14142 | 1993-09-01 | 2 | 2 |
| FW | GE | 1108 | Nitrogen Ammonia Dissolv | mg/L | 0.00600 | < | 0.00500 | 0.00600 | 0.00550 | 0.00000 | 1993-09-01 | 2 | 1 |
| FW | GE | 1109 | Nitrate(NO3) + Nitrite(N(| mg/L | 0.36000 | | 0.31000 | 0.33500 | 0.33500 | 0.03536 | 1993-09-01 | 2 | 2 |
| FW | GE | 1110 | Nitrate (NO3) Dissolved | mg/L | 0.36000 | | 0.31000 | 0.33500 | 0.33500 | 0.03536 | 1993-09-01 | 2 | 2 |
| FW | GE | 1111 | Nitrogen - Nitrite Disso | mg/L | 0.00500 | < | 0.00500 | | 0.00500 | | 1993-09-01 | 2 | 0 |
| FW | GE | 1113 | Nitrogen (Kjeldahl) Tota | mg/L | 0.04 | < | 0.04 | | 0.04 | | 1992-12-08 | 1 | 0 |
| FW | GE | 1114 | Nitrogen Total Dissolved | mg/L | 0.31 | | 0.31 | 0.31 | 0.31 | 0.00 | 1992-12-08 | 1 | 1 |
| FW | GE | 1121 | Sulfate Dissolved | mg/L | 2.20000 | | 2.00000 | 2.10000 | 2.10000 | 0.14142 | 1993-09-01 | 2 | 2 |
| FW | GE | AG-D | Silver Dissolved | mg/L | 0.01000 | < | 0.01000 | | 0.01000 | | 1993-09-01 | 2 | 0 |
| FW | GE | AG-T | Silver Total | mg/L | 0.03000 | < | 0.01000 | | 0.02000 | | 1993-09-01 | 2 | 0 |
| FW | GE | AL-D | Aluminum Dissolved | mg/L | 0.02000 | < | 0.02000 | | 0.02000 | | 1993-09-01 | 2 | 0 |
| FW | GE | AL-T | Aluminum Total | mg/L | 0.06000 | < | 0.02000 | | 0.04000 | | 1993-09-01 | 2 | 0 |
| FW | GE | AS-D | Arsenic Dissolved | mg/L | 0.04000 | < | 0.00100 | 0.04000 | 0.02050 | 0.00000 | 1993-09-01 | 2 | 1 |
| FW | GE | AS-T | Arsenic Total | mg/L | 0.00 | < | 0.00 | | 0.00 | | 1993-09-01 | 2 | 0 |
| FW | GE | BD | Boron Dissolved | mg/L | 0.00800 | < | 0.00800 | | 0.00800 | | 1993-09-01 | 2 | 0 |
| FW | GE | BT | Boron Total | mg/L | 0.043 | < | 0.040 | 0.043 | 0.042 | 0.000 | 1993-09-01 | 2 | 1 |
| FW | GE | BA-D | Barium Dissolved | mg/L | 0.00200 | < | 0.00100 | 0.00200 | 0.00150 | 0.00000 | 1993-09-01 | 2 | 1 |
| FW | GE | BA-T | Barium Total | mg/L | 0.00100 | < | 0.00100 | | 0.00100 | | 1993-09-01 | 2 | 0 |
| | | | | | | | | | | | | | |

Outliers are not included, and results less than or greater than detection limits have been excluded in Mean and Standard Deviation.

Environmental Monitoring System Historical Statistics Report

Name: FVGMP STAVE LAKE ROAD, MISSION

Page 3 of 4

Monitoring Location: E217927

Office: LOWER MAINLAND

Establishing Agency: WMB-Groundwater

Watershed Code:

Latitude: 049:15:27 Longitude: 122:14:37

| Location Type: | WELL | |
|------------------|--------------|------------|
| No. Samples: | 2 | |
| First Collection | Date: | 1992-12-08 |
| Most Recent Coll | ection Date: | 1993-09-01 |

| | mple Desc. | Parameter | Units | Maximum | Minimum | Mean | Median | Standard Deviation | Latest Date | Total Obs. | Used Obs. |
|----|---------------|-----------------------------|-------|-----------|---------|---------|---------|-----------------------|-------------|---------------|--------------|
| FW | GE | BE-D Beryllium Dissolved | mg/L | 0.00100 < | 0.00100 | | 0.00100 | | 1993-09-01 | 2 | 0 |
| FW | GE | BE-T Beryllium Total | mg/L | 0.00100 < | 0.00100 | | 0.00100 | | 1993-09-01 | 2 | 0 |
| FW | GE | BI-D Bismuth Dissolved | mg/L | 0.02000 < | 0.02000 | | 0.02000 | | 1993-09-01 | 2 | 0 |
| FW | GE | BI-T Bismuth Total | mg/L | 0.02000 < | 0.02000 | | 0.02000 | | 1993-09-01 | 2 | 0 |
| FW | GE | CA-D Calcium Dissolved | mg/L | 6.78000 | 6.70000 | 6.74000 | 6.74000 | 0.05657 | 1993-09-01 | 2 | 2 |
| FW | GE | CA-T Calcium Total | mg/L | 6.90000 | 6.53000 | 6.71500 | 6.71500 | 0.26163 | 1993-09-01 | 2 | 2 |
| FW | GE | CD-D Cadmium Dissolved | mg/L | 0.00200 < | 0.00200 | | 0.00200 | | 1993-09-01 | 2 | 0 |
| FW | GE | CD-T Cadmium Total | mg/L | 0.00200 < | 0.00200 | | 0.00200 | | 1993-09-01 | 2 | 0 |
| FW | GE | CO-D Cobalt Dissolved | mg/L | 0.00300 < | 0.00300 | | 0.00300 | | 1993-09-01 | 2 | 0 |
| FW | GE | CO-T Cobalt Total | mg/L | 0.00400 < | 0.00300 | | 0.00350 | | 1993-09-01 | 2 | 0 |
| FW | GE | CR-D Chromium Dissolved | mg/L | 0.00200 < | 0.00200 | | 0.00200 | | 1993-09-01 | 2 | 0 |
| FW | GE | CR-T Chromium Total | mg/L | 0.00200 < | 0.00200 | | 0.00200 | | 1993-09-01 | 2 | 0 |
| FW | GE | CU-D Copper Dissolved | mg/L | 0.05400 | 0.00300 | 0.02850 | 0.02850 | 0.03606 | 1993-09-01 | 2 | 2 |
| FW | GE | CU-T Copper Total | mg/L | 0.05500 | 0.00800 | 0.03150 | 0.03150 | 0.03323 | 1993-09-01 | 2 | 2 |
| FW | GE | FE-D Iron Dissolved | mg/L | 0.01200 < | 0.00300 | | 0.00750 | | 1993-09-01 | 2 | 0 |
| FW | GE | FE-T Iron Total | mg/L | 0.07000 < | 0.02300 | | 0.04650 | | 1993-09-01 | 2 | 0 |
| FW | GE | KD Potassium Dissolved | mg/L | 0.40000 < | 0.40000 | | 0.40000 | | 1993-09-01 | 2 | 0 |
| FW | GE | KT Potassium Total | mg/L | 0.60000 < | 0.40000 | 0.60000 | 0.50000 | 0.00000 | 1993-09-01 | 2 | 1 |
| FW | GE | MG-D Magnesium Dissolved | mg/L | 0.75000 | 0.75000 | 0.75000 | 0.75000 | 0.00000 | 1993-09-01 | 2 | 2 |
| FW | GE | MG-T Magnesium Total | mg/L | 0.73000 | 0.71000 | 0.72000 | 0.72000 | 0.01414 | 1993-09-01 | 2 | 2 |
| FW | GE | MN-D Manganese Dissolved | mg/L | 0.00200 < | 0.00200 | | 0.00200 | | 1993-09-01 | 2 | 0 |
| FW | GE | MN-T Manganese Total | mg/L | 0.00200 < | 0.00200 | | 0.00200 | | 1993-09-01 | 2 | 0 |
| FW | GE | MO-D Molybdenum Dissolved | mg/L | 0.02000 < | 0.00400 | 0.02000 | 0.01200 | 0.00000 | 1993-09-01 | 2 | 1 |
| FW | GE | MO-T Molybdenum Total | mg/L | 0.00400 < | 0.00400 | | 0.00400 | | 1993-09-01 | 2 | 0 |
| FW | GE | NA-D Sodium Dissolved | mg/L | 1.95000 | 1.90000 | 1.92500 | 1.92500 | 0.03536 | 1993-09-01 | 2 | 2 |
| FW | GE | NA-T Sodium Total | mg/L | 2.76000 | 1.80000 | 2.28000 | 2.28000 | 0.67882 | 1993-09-01 | 2 | 2 |
| FW | GE | NI-D Nickel Dissolved | mg/L | 0.00800 < | 0.00800 | | 0.00800 | | 1993-09-01 | 2 | 0 |
| FW | GE | NI-T Nickel Total | mg/L | 0.01000 < | 0.01000 | | 0.01000 | | 1993-09-01 | 2 | 0 |
| FW | GE | PD Phosphorus Total Dissolv | mg/L | 0.04000 < | 0.04000 | | 0.04000 | | 1993-09-01 | 2 | 0 |
| FW | GE | PT Phosphorus Total | mg/L | 0.04000 < | 0.04000 | | 0.04000 | | 1993-09-01 | 2 | 0 |

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Environmental Monitoring System Historical Statistics Report

Name: FVGMP STAVE LAKE ROAD, MISSION

Page 4 of 4

Monitoring Location: E217927

Office: LOWER MAINLAND

Establishing Agency: WMB-Groundwater

Watershed Code:

Latitude: 049:15:27 Longitude: 122:14:37

| Location Type: | WELL | |
|------------------|--------------|------------|
| No. Samples: | 2 | |
| First Collection | Date: | 1992-12-08 |
| Most Recent Coll | ection Date: | 1993-09-01 |

| | Units | Maximum | | Minimum | Mean | Median | Standard | Latest Date | Total | |
|--------------------------|---|---|--|--|--|--|---|--|---|---|
| с. | | | | | | | Deviation | | Obs. | Obs. |
| PB-D Lead Dissolved | mg/L | 0.001 | < | 0.001 | | 0.001 | | 1993-09-01 | 2 | 0 |
| PB-T Lead Total | mg/L | 0.014 | < | 0.001 | 0.014 | 0.008 | 0.000 | 1993-09-01 | 2 | 1 |
| SD Sulfur Dissolved | mg/L | 0.82000 | | 0.73000 | 0.77500 | 0.77500 | 0.06364 | 1993-09-01 | 2 | 2 |
| ST Sulfur Total | mg/L | 0.80000 | | 0.78000 | 0.79000 | 0.79000 | 0.01414 | 1993-09-01 | 2 | 2 |
| SB-D Antimony Dissolved | mg/L | 0.01500 | < | 0.01500 | | 0.01500 | | 1993-09-01 | 2 | 0 |
| SB-T Antimony Total | mg/L | 0.01500 | < | 0.01500 | | 0.01500 | | 1993-09-01 | 2 | 0 |
| SE-D Selenium Dissolved | mg/L | 0.03000 | < | 0.03000 | | 0.03000 | | 1993-09-01 | 2 | 0 |
| SE-T Selenium Total | mg/L | 0.00500 | < | 0.00500 | | 0.00500 | | 1993-09-01 | 2 | 0 |
| SI-D Silicon Dissolved | mg/L | 6.53000 | | 6.15000 | 6.34000 | 6.34000 | 0.26870 | 1993-09-01 | 2 | 2 |
| SI-T Silicon Total | mg/L | 6.32000 | | 6.00000 | 6.16000 | 6.16000 | 0.22627 | 1993-09-01 | 2 | 2 |
| SN-D Tin Dissolved | mg/L | 0.02000 | < | 0.02000 | | 0.02000 | | 1993-09-01 | 2 | 0 |
| SN-T Tin Total | mg/L | 0.02000 | < | 0.02000 | | 0.02000 | | 1993-09-01 | 2 | 0 |
| SR-D Strontium Dissolved | mg/L | 0.02200 | | 0.02200 | 0.02200 | 0.02200 | 0.00000 | 1993-09-01 | 2 | 2 |
| SR-T Strontium Total | mg/L | 0.02400 | | 0.02100 | 0.02250 | 0.02250 | 0.00212 | 1993-09-01 | 2 | 2 |
| TE-D Tellerium Dissolved | mg/L | 0.02000 | < | 0.02000 | | 0.02000 | | 1993-09-01 | 2 | 0 |
| TE-T Tellurium Total | mg/L | 0.02000 | < | 0.02000 | | 0.02000 | | 1993-09-01 | 2 | 0 |
| TI-D Titanium Dissolved | mg/L | 0.00300 | < | 0.00300 | | 0.00300 | | 1993-09-01 | 2 | 0 |
| TI-T Titanium Total | mg/L | 0.00300 | < | 0.00300 | | 0.00300 | | 1993-09-01 | 2 | 0 |
| TL-D Thallium Dissolved | mg/L | 0.02000 | < | 0.02000 | | 0.02000 | | 1993-09-01 | 2 | 0 |
| TL-T Thallium Total | mg/L | 0.03000 | < | 0.02000 | | 0.02500 | | 1993-09-01 | 2 | 0 |
| VD Vanadium Dissolved | mg/L | 0.00300 | < | 0.00300 | | 0.00300 | | 1993-09-01 | 2 | 0 |
| VT Vanadium Total | mg/L | 0.00300 | < | 0.00300 | | 0.00300 | | 1993-09-01 | 2 | 0 |
| ZN-D Zinc Dissolved | mg/L | 0.02100 | < | 0.00000 | 0.01550 | 0.01000 | 0.00778 | 1993-09-01 | 3 | 2 |
| ZN-T Zinc Total | mg/L | 0.03000 | | 0.01100 | 0.02050 | 0.02050 | 0.01344 | 1993-09-01 | 2 | 2 |
| ZR-D Zirconium Dissolved | mg/L | 0.00300 | < | 0.00300 | | 0.00300 | | 1993-09-01 | 2 | 0 |
| ZR-T Zirconium Total | mg/L | 0.00300 | < | 0.00300 | | 0.00300 | | 1993-09-01 | 2 | 0 |
| | <pre>c. PB-D Lead Dissolved PB-T Lead Total PB-T Lead Total SD Sulfur Dissolved ST Sulfur Total SB-D Antimony Dissolved SB-T Antimony Total SE-D Selenium Dissolved SE-T Selenium Total SI-D Silicon Dissolved SI-T Silicon Total SN-D Tin Dissolved SN-T Tin Total SR-D Strontium Dissolved SR-T Strontium Dissolved TE-T Tellurium Total TL-D Titanium Dissolved TL-T Thallium Dissolved TL-T Thallium Total VD Vanadium Dissolved ZN-T Zinc Dissolved ZN-T Zinc Total </pre> | c.PB-DLead Dissolvedmg/LPB-TLead Totalmg/LSDSulfur Dissolvedmg/LSTSulfur Totalmg/LSB-DAntimony Dissolvedmg/LSB-DAntimony Totalmg/LSE-TSelenium Dissolvedmg/LSE-TSelenium Totalmg/LSI-DSilicon Dissolvedmg/LSI-TSilicon Totalmg/LSN-DTin Dissolvedmg/LSN-DTin Dissolvedmg/LSR-DStrontium Dissolvedmg/LSR-TStrontium Totalmg/LTE-DTellerium Dissolvedmg/LTI-DTitanium Totalmg/LTL-DThallium Dissolvedmg/LTL-TThallium Dissolvedmg/LTL-TTotalmg/LVDVanadium Dissolvedmg/LVTVanadium Dissolvedmg/LZN-DZinc Dissolvedmg/LZN-TZinc Totalmg/LZR-DZirconium Dissolvedmg/L | c.c.PB-DLead Dissolvedmg/L0.001PB-TLead Totalmg/L0.014SDSulfur Dissolvedmg/L0.82000STSulfur Totalmg/L0.80000SB-DAntimony Dissolvedmg/L0.01500SB-TAntimony Totalmg/L0.01500SE-DSelenium Dissolvedmg/L0.03000SE-TSelenium Totalmg/L0.00500SI-DSilicon Dissolvedmg/L6.53000SI-TSilicon Totalmg/L0.02000SN-DTin Dissolvedmg/L0.02000SN-TTin Totalmg/L0.02000SR-DStrontium Dissolvedmg/L0.02000SR-TStrontium Totalmg/L0.02000TI-DTellerium Totalmg/L0.02000TI-DTitanium Totalmg/L0.02000TL-DThallium Dissolvedmg/L0.02000TL-DThallium Dissolvedmg/L0.02000TL-TThallium Dissolvedmg/L0.03000VDVanadium Dissolvedmg/L0.03000VTVanadium Dissolvedmg/L0.00300VTVanadium Totalmg/L0.00300ZL-TTinc Totalmg/L0.02100ZR-DZinc Dissolvedmg/L0.03000ZR-DZinc noinu Dissolvedmg/L0.03000 | c.Bench marketpB-DLead Dissolvedmg/L0.001 <PB-TLead Totalmg/L0.014 <SDSulfur Dissolvedmg/L0.82000STSulfur Totalmg/L0.80000SB-DAntimony Dissolvedmg/L0.01500 <SB-TAntimony Totalmg/L0.01500 <SE-DSelenium Dissolvedmg/L0.03000 <SE-TSelenium Totalmg/L0.00500 <SI-DSilicon Dissolvedmg/L6.32000SN-DTin Dissolvedmg/L0.02000 <SN-DTin Dissolvedmg/L0.02000 <SR-DStrontium Dissolvedmg/L0.02200SR-TStrontium Dissolvedmg/L0.02200SR-TStrontium Totalmg/L0.02000 <TE-DTellerium Dissolvedmg/L0.02000 <TI-DTitanium Totalmg/L0.02000 <TI-TTitanium Dissolvedmg/L0.03000 <TI-DTitanium Totalmg/L0.03000 <TI-DThallium Dissolvedmg/L0.03000 <VDVanadium Dissolvedmg/L0.03000 <VDVanadium Dissolvedmg/L0.02100 <ZN-TZinc Totalmg/L0.03000ZN-TZinc Totalmg/L0.03000ZN-DZinc Totalmg/L0.03000ZN-DZinc Totalmg/L0.03000ZN-DZinc Totalmg/L0.03000ZN-TZinc To | c. maximum matrix PB-D Lead Dissolved mg/L 0.001 < | c. number PB-D Lead Dissolved mg/L 0.001 0.001 PB-T Lead Total mg/L 0.014 0.001 0.014 SD Sulfur Dissolved mg/L 0.82000 0.73000 0.77500 ST Sulfur Total mg/L 0.80000 0.78000 0.79000 SB-T Antimony Dissolved mg/L 0.01500 0.01500 SE-D Selenium Dissolved mg/L 0.00500 0.00500 SE-D Selenium Total mg/L 6.53000 6.15000 6.34000 SI-T Silicon Dissolved mg/L 0.02000 0.02000 SN-D Tin Dissolved mg/L 0.02000 0.02200 SN-T Tin Total mg/L 0.02000 0.02200 SR-D Strontium Dissolved mg/L 0.02200 0.02200 SR-D Strontium Total mg/L 0.02000 0.02200 | C Number PB-D Lead Dissolved mg/L 0.001 0.001 0.001 PB-T Lead Total mg/L 0.014 0.001 0.014 0.008 SD Sulfur Dissolved mg/L 0.82000 0.73000 0.77500 0.77500 ST Sulfur Total mg/L 0.80000 0.78000 0.79000 0.79000 SB-D Antimony Dissolved mg/L 0.01500 0.01500 0.01500 0.01500 SB-D Selenium Dissolved mg/L 0.03000 0.03000 0.03000 SE-D Selenium Total mg/L 0.00500 0.00500 0.00500 SI-D Silicon Dissolved mg/L 6.32000 6.00000 6.16000 6.16000 SI-T Silicon Total mg/L 0.02000 0.02200 0.02200 0.02200 SN-D Tin Dissolved mg/L 0.02000 0.02200 0.02200 0.02200 SR-T Strontium Total mg/L 0.02000 | Desc Deviation PB-D Lead Dissolved mg/L 0.001 0.001 PB-T Lead Total mg/L 0.014 0.001 0.014 0.000 SD Sulfur Dissolved mg/L 0.82000 0.73000 0.77500 0.77500 0.06364 ST Sulfur Total mg/L 0.8000 0.78000 0.79000 0.0144 SB-D Antimony Dissolved mg/L 0.01500 0.01500 0.01500 SB-T Antimony Total mg/L 0.03000 0.03000 0.03000 SB-T Selicon Dissolved mg/L 0.03000 0.00500 0.26870 SI-T Silicon Dissolved mg/L 6.53000 6.15000 6.16000 0.22627 SN-T Tin Total mg/L 0.02000 0.02000 0.02200 SR-D Strontium Dissolved mg/L 0.02000 0.02200 0.02200 SR-D Terontium Total | number Deviation PB-D Lead Dissolved ng/L 0.001 < | Destination Deviation Deviation Obs. PB-T Lead Dissolved mg/L 0.001 < |

Outliers are not included, and results less than or greater than detection limits have been excluded in Mean and Standard Deviation.

APPENDIX H

SCOTT RESEARCH SERVICES INC. REPORT



SCOTT RESOURCE SERVICES INC.

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March 26, 2012

Our file: 822.04 Your file: 3131

Ms. Kathy C. Tixier, *P. Eng.* Senior Hydrogeologist Piteau Associates Engineering Ltd. #215 – 260 W. Esplanade North Vancouver, BC, V7M 3G7

Re: Overview environmental assessment of stream, fish and wildlife habitat resources in the vicinity of a proposed groundwater development in the Miracle Valley, north of Hatzic Lake, B.C.

1.0 INTRODUCTION

Per the request of Piteau Associates Engineering Ltd. (Piteau), Scott Resource Services Inc. (SRS) has prepared an overview level environmental assessment of stream, fish and wildlife resources in the Miracle Valley north of Hatzic Lake, B.C. (Attachment 1). The Miracle Valley aquifer is being investigated as a potential source of municipal water supply.

SRS's overview environmental assessment methodology included background research and brief ground-truthing assessments of the watercourses in the area to determine potential impacts to the creeks and associated fish and wildlife habitats.

Specifically, a detailed review of creeks located in the southern part of the aquifer, including Lagace Creek, Belcharton Creek, Durieu Creek, Oru Creek and Seux Brook, and the northern part of the aquifer, including Marino Creek and an unnamed tributary to Cascade Creek was conducted (Attachment 2).

The review identified watercourse attributes and fish habitats that could be potentially affected by groundwater extraction from the aquifer. A review of aquatic species at risk within the area was also conducted.

SRS further provides potential ecological and fish habitat implications associated with a reduction in seasonal water flows.

2.0 METHODS

An electronic search of the Ministry of Environment's (MOE) Fisheries Inventory Summary System (FISS) and the Fraser Valley Regional District's Habitat Atlas (FVHA) was conducted. A search for species at risk was conducted through the British Columbia Conservation Data Centre (BCCDC). In addition, a review of SRS's archives for projects conducted in the vicinity of the aquifer and background information pertaining to the area was gathered.

A brief field reconnaissance was also completed.

3.0 WATERCOURSE & BIOPHYSICAL ASSESSMENT

3.1 Biophysical assessment

Four watercourses in the southern region of the aquifer, Belcharton Creek, Durieu Creek, Oru Creek and Seux Brook, were identified by Piteau as having the potential to be negatively affected by groundwater extraction. All of these watercourses are tributaries to Lagace Creek which drains to Hatzic Lake, which in turn drains to the Fraser River. In addition, sections of the northern region of the aquifer, including Marino Creek and an unnamed tributary to Cascade Creek were identified as streams that may be negatively affected by groundwater extraction in the area. Streams in the northern region of the aquifer drain into Stave Lake. An annotated orthophotograph showing the general location of these watercourses is included in Attachment 2.

Magwood (2004) found that in 2000 most of the land in Miracle Valley was forested (74%), while some was residential (16%) and agricultural (7%) and the remainder (3%) was for other uses (ex. roads, wetlands).

The Miracle Valley aquifer is a sand and gravel aquifer that is recharged from water infiltration sourced from the overlying creeks and precipitation (Piteau, 2007; PHCL, 2003). The aquifer is confined (overlain by a low permeability layer) and the depth to the top of the aquifer is approximately 38 m (PHCL, 2003). Research by Piteau (2007) indicates that flow to Belcharton, Durieu, Oru and Seux are predominantly sourced by groundwater discharge. Hydrographs of Lagace and Belcharton Creek indicate that the average stream flows in the area are highest from November to January and are lowest from June to September (Rood and Hamilton, 1995).

3.2 Fish and fish habitat

3.2.1 Lagace Creek

Lagace Creek is the main creek entering Hatzic Lake with a drainage area of approximately 32 km² (Rood and Hamilton, 1995). From FISS records, Lagace Creek is known to support populations of cutthroat trout (*Onchorhynchus clarki*), rainbow trout and steelhead trout (*O. mykiss*), coho salmon (*O. kisutch*), chum salmon (*O. keta*), three-spined stickleback (*Gasterosteus spp.*), lamprey (*Lampetra spp.*) and sculpin (*Cottus spp.*). A 1999 report by

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Fisheries and Oceans Canada (DFO) indicated that rearing potential is good in Lagace Creek and its tributaries.

In August and September of 2005, SRS conducted a fish salvage in Lagace Creek, and reported findings of cutthroat trout, rainbow trout, coho salmon, three-spined stickleback, lamprey and sculpin. In particular there were large numbers of coho salmon (9,343 individuals), rainbow trout (1,011 individuals) and sculpin (2,693 individuals) that were salvaged and relocated during this period. There are no reported physical barriers between Lagace Creek and the four tributaries in the south. All the southern streams – Belcharton, Durieu, Oru and Seux are low gradient and fish-bearing (Scott, 2011).

Lagace Creek is diked for 4 km upstream of Hatzic Lake. It is subject to active gravel and debris extraction and dredging for flood and erosion protection. Low gradient reaches within the Hatzic – Miracle Valley area are subject to aggradation, due to landslide and other hillslope instabilities in the headwater reaches.

Previous studies have suggested that domestic wells in the area and extraction of water for agricultural irrigation have altered the hydrology of the creek and its tributaries (DFO, 1999; NHC, 2005).

3.2.1.1 Belcharton Creek

Belcharton Creek is known to support populations of coho and chum salmon and has been identified as high quality rearing habitat. It is also reported to support populations of cutthroat trout (DFO, 1999). This finding is further supported by the FVHA that has delineated Belcharton Creek as fish bearing habitat. The first kilometre of Belcharton Creek (from its confluence with Lagace Creek), is reported to be one of the largest habitat contributors for salmonid spawning to the Hatzic Lake system (DFO, 1999).

Belcharton Creek has been identified as a major spawning location for both chum salmon and coho salmon (FISS, 2011). Chum salmon migration to Belcharton Creek typically begins in early October with spawning occurring from mid-October to late December. Coho salmon migration to Belcharton Creek typically begins in early November with spawning occurring between mid-November to as late as January.

SRS has completed assessments in Belcharton Creek, and has found a year-round population of coho salmon in various life stages (Scott, 2011; SRS, 2005). Belcharton Creek has also been identified as important cutthroat trout habitat (DFO, 1999).

Precision Identification Biological Consultants (PIBC) classified Belcharton Creek as an endangered stream in 1998. Belcharton was reported to have water quality problems, impacts from logging and "other impacts" (i.e. agricultural/ urban impacts, anthropogenic barriers and cumulative effects of these impacts). Historic records indicate that Belcharton Creek has suffered from poor water extraction management and 1995 records indicate that the summer water extraction from Belcharton Creek was extreme with water licenses amounting to two times the mean August flow for the creek (Rood and Hamilton; 1995).

There are also reports that baseflows in Belcharton Creek were altered due to an active rock quarry operation located near the headwaters of the creek (DFO, 1999).

3.2.1.2 Durieu Creek, Oru Creek and Seux Brook

A search of FISS did not return any records for Durieu Creek, Oru Creek or Seux Brook. However, both Durieu and Oru Creek were delineated as fish bearing habitat on the FVHA. Seux Brook was classified as having unknown fish presence on the FVHA.

A 1999 DFO report classified Durieu Creek as important coho salmon habitat (DFO, 1999). Given the low gradient habitat and the lack of physical barriers between Lagace Creek and its tributaries, there is a high likelihood that coho salmon, chum salmon and cutthroat trout are present in all three streams. All streams are located in close proximity to one another (within 1.5 km) and have very similar stream gradients and adjoining riparian vegetation; thus the stream hydrology of Durieu, Oru and Seux are likely similar to that of Belcharton Creek. However, confirmation of this finding would require additional and more extensive ground-truthing.

PIBC (1998) classified Durieu Creek, Oru Creek and Seux Brook as threatened streams due to agricultural and urban habitat impacts.

3.2.2 Marino Creek

Marino Creek is predominantly spring fed and flows north into Stave Lake. According to the FVHA the creek is fish bearing from its headwaters to Stave Lake. The low gradient reaches of the creek would provide good habitat for cutthroat and rainbow trout. Stave Lake is a non-anadromous lake.

According to Madrone Environmental Services Ltd. (MES) there are currently two water licenses on Marino Creek including one that is held by the active rock quarry (as referenced under Belcharton Creek).

3.2.3 Cascade Creek

Cascade Creek is a large tributary to Stave Lake. From FISS records, Cascade Creek is known to support populations of cutthroat trout, rainbow trout, kokanee salmon (*Oncorhynchus nerka*), Dolly Varden char (*Salvelinus malma malma*) and sculpin.

Golder Associates Ltd. (2008), classified Cascade Creek as a "fairly wide and low gradient stream" that has been subject to channel erosion and destabilization. SRS used satellite imagery to estimate the average channel width to be 20 m in the vicinity of the project area. PIBC's study (1998) classified Cascade Creek as an endangered stream. Cascade Creek was reported to have been impacted by riparian removal, effects of urbanization and "other impacts".

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3.2.3.1 Tributary to Cascade Creek

According to the FVHA, the tributary to Cascade Creek is delineated as fish bearing habitat on the northern end of the stream and is delineated as having unknown fish presence on the southern end of the stream.

The tributary is low gradient and fish-bearing in the area highlighted by Piteau as a stream of interest. Given the lack of physical barriers between Cascade Creek and the tributary, the tributary is considered by default to be a fish bearing stream reach with a strong likelihood that salmonids (trout and char) are present.

3.3 Wildlife and Species at Risk

A search of the BCCDC map indicated that Pacific water shrew (*Sorex bendirii*) (PWS) was observed in 2000 in the Miracle Valley area in close proximity to Lagace Creek and its tributaries. There was also a sighting in 1992 just north of Cascade Creek, near Stave Lake. PWS are usually associated with riparian areas of moist forests and are found within one kilometre of a water body (Bianchini, 2010). PWS are endangered and red-listed by the province (BCCDC, 2012). The riparian area overlying the Miracle Valley Aquifer is considered suitable habitat for PWS.

In addition, a search of BCCDC revealed that red-legged frog (*Rana aurora*) was observed in 2006 in the Miracle Valley area within the high water mark of Lagace Creek. Critical habitat for the species include temporary and permanent breeding ponds and the species is red-listed by the province.

Multiple sightings of terrestrial Oregon Forestsnail (*Allogona townsendiana*) in 2005 and 2006 were reported within the area (BCCDC, 2012).

Although there were no recorded observations, the area overlying the Miracle Valley Aquifer was also identified by BCCDC as suitable habitat for Emma's dancer (*Argia emma*), a species of dragonfly that breeds in riffle habitats of streams. This species is of special concern (blue-listed) in the province.

The area would also provide suitable habitat for Pacific waterleaf (*Hydrophyllum tenuipes*), a plant that has been red-listed by the BCCDC (2012). This species is typically found in lowland moist forests and streambanks (Bianchini, 2010).

4.0 CONCLUSIONS

All of the southern streams are predominantly sourced by groundwater discharge (Piteau, 1994). Groundwater aquifers in the area tend to recharge during the fall and winter rains. Current sources of groundwater recharge also include water from streams (Piteau, 2007; PPHCL, 2003). Habitat quality for fish and other aquatic species is often dependent on a supply of cold and clean groundwater (FVRD, 2010).

SRS's overview investigation of existing literature and known habitat constraints suggests that existing and historic diversion for waterworks, irrigation or industrial use have had an effect on Belcharton Creek, and evidence suggests that use of the water is over-subscribed compared to the estimated available flows (Magwood, 2004). Summer water use in Belcharton Creek has been rated as extreme and FISS has described this water use and diversion as a constraint for fisheries production.

SRS anticipates that Seux Brook, Oru Creek and Durieu Creek would have similar hydrological constraints as Belcharton Creek.

Drawdowns in the water supply to the streams has the potential to affect surface water availability which can be a major hazard to fish and aquatic species especially when it comes to maintaining minimal flows over spawning beds within a creek. While peak water flows in the winter would to a large degree mitigate the potential effects on salmon eggs and alevins in the gravel, there are significant risks associated with drawdowns and the effect on available rearing habitat for coho salmon and trout fry and smolts. There is also significant risk of trout redd dewatering in late spring.

Maintaining baseflows in the creek is also important for access to off-channel habitat that may only be accessible during high flows (Douglas, 2006). Reductions in groundwater can also influence the thermal refuge for fish by causing earlier cooling and ice formation in streams in the winter months and faster warming in the summer months. Extremes in temperature can also increase fish mortality and stress and decrease the carry capacity of the habitat (Douglas, 2006).

Reductions in groundwater volume can influence water quality by reducing the supply of nutrients to streams. Groundwater extraction can also reduce the amount of water available for riparian vegetation which can have negative ecological effects on the streams (Douglas, 2006). Hancock (2002) found that groundwater extraction lowers the residence time of water in the hyporheic layer (located between the surface water and groundwater aquifer) and can influence biological activity. The effect of groundwater extraction on riparian vegetation would be very difficult to quantify.

Given the existing effects of reduced baseflow within Belcharton Creek, and presumed similar effects in Seux Brook, Durieu Creek and Oru Creek, SRS anticipates that obtaining environmental approvals to withdraw additional water from the Miracle Valley aquifer will be difficult, and would only be granted following adequate:

- modeling to quantify the effect;
- biophysical assessments to determine existing baseline environmental resources; and,
- mitigation or compensation to offset the quantified effect on the relevant species.

Based on SRS's experience with the Bevan Wells project in Abbotsford, it is probable that the proposed water withdrawals would be of sufficient enough size to trigger a requirement for the project to be reviewed under the British Columbia Environmental Assessment Act (BCEAA). Since the works have the potential to affect fish habitat, DFO would be a Federal Authority with jurisdiction.

Unless adequate mitigation could be developed, it seems unlikely that DFO would approve a harmful alteration disruption or destruction (HADD) of fish habitat in Belcharton Creek due to the important habitat available for coho salmon. Were DFO to grant approval for a HADD, then that would require an Authorization per Section 35(2) of the federal *Fisheries Act*. The requirement for DFO to issue an Authorization is an automatic trigger for the project to be screened through the auspices of the *Canadian Environmental Assessment Act* (CEAA).

It is difficult to ascertain what mitigation might be required at this preliminary stage of the investigation, but concepts associated with water withdrawals and reservoir storage during periods of the year when recharge is high and creek flows are high may be acceptable to the environmental regulatory agencies. Another alternative might be to augment stream flows with water transported from other drainages or surface reservoirs to offset effects associated with the withdrawals from the confined aquifer. Feasibility of such a mitigatory approach would have to be reviewed by the owner and proponent as alternate water supply systems may prove to be more practical.

In closing, there is going to be a significant environmental regulatory review process involved with any proposed groundwater extraction in the Miracle Valley. Efforts by the owner's project team to satisfy the regulatory review would also be substantial, because it is unlikely that DFO would support a groundwater withdrawal that significantly reduces existing baseflows, since these baseflows already appear to be below historic levels. Perhaps a water management strategy for the entire valley could be developed to mitigate effects and at the same time allow for additional water extraction.

5.0 RECOMMENDATIONS

- Undertake stream modeling to determine peak and base flows within the affected streams in the Miracle Valley.
- Investigate the feasibility of developing a water supply design configuration that withdraws water and stores water when recharge is high and stream discharge is high, avoiding withdrawals during low water periods.
- Consider augmenting flow to streams during low water periods as a mitigatory measure.
- Concurrent with advancement of the project to a conceptual or preliminary design (and decision to seek project environmental approvals), undertake a comprehensive baseline investigation of fish populations and species at risk populations (predominantly aquatic) within the project area.

6.0 CLOSURE

I trust this is the information you require at this time. If you have any questions please do not hesitate to phone the undersigned to discuss the contents of this overview environmental assessment.

Regards,

SCOTT RESOURCE SERVICES INC.

anne Rutto

Anne Rutherford, *BSc*, *EPt* Environmental Technician

David E. Aleufeld

David E. Neufeld, *R.P.Bio*. Senior Project Manager

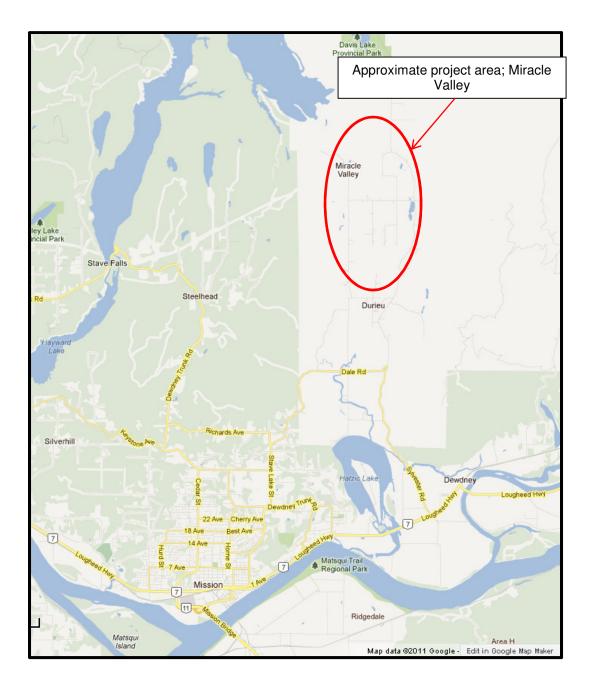
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Attachment 1

Site location map



Attachment 2

Annotated orthophotograph

